

Civil Engineering Design Report

New High School for Schofields and Tallawong

Prepared for NSW Department of Education / 31 January 2025

241460

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1.0 Introduction

This Civil Engineering Design Report has been prepared to support a Review of Environmental Factors (REF) for the NSW Department of Education (DoE) for the construction and operation of the new Schofields - Tallawong High School (the activity).

The purpose of the REF is to assess the potential environmental impacts of the activity prescribed by *State Environmental Planning Policy (Transport and Infrastructure) 2021* (T&I SEPP) as "development permitted without consent" on land carried out by or on behalf of a public authority under Part 5 of the *Environmental Planning and Assessment Act 1979* (EP&A Act). The activity is to be undertaken pursuant to Chapter 3, Part 3.4, Section 3.37A of the T&I SEPP.

This document has been prepared in accordance with the *Guidelines for Division 5.1 assessments* (the Guidelines) by the Department of Planning, Housing and Infrastructure (DPHI).

1.1 Guidance documents

The following documents have been reviewed and referenced in preparing this report:

- Blacktown City Council (BCC) Development Control Plan (DCP), 2015;
- Blacktown City Council (BCC) Water Sensitive Urban Design (WSUD) Developer Handbook MUSIC Modelling and Design Guide, 2020;
- Blacktown City Council (BCC) Engineering Guide for Development (EGD), 2005;
- Upper Parramatta River Catchment Trust (UPRCT) On-Site Stormwater Detention Handbook Fourth Edition, December 2005;
- Blue Book Managing Urban Stormwater Soils and Construction (Landcom NSW);
- NSW Department of Education Educational Facilities Standards and Guidelines;
- Australian Rainfall and Runoff 2019;
- Before You Dig Australia (BYDA) site investigation (Ref no. 3558608) dated 24 October 2024;

Design inputs by others:

- New High School for Schofields and Tallawong Geotechnical Investigation (Ref no. PSM4693-012L Rev4) by PSM dated 11 December 2024;
- Schofields Tallawong High School, Site 1, Guntawong Road Bulk Earthwork Specification (Ref no. PSM4693-013S DRAFT) by PSM dated 21 November 2024;
- Detailed Site Investigation Report, New High School for Schofields and Tallawong (Ref no. 67774-162496) by JBS&G dated 5 December 2024;
- Remedial Action Plan, Proposed Tallawong High School (Ref no. 67774-162922) by JBS&G dated 5 December 2024;
- Draft Preliminary Long Term Environmental Management Plan, Proposed Tallawong High School (Ref no. 67774-162926) by JBS&G dated 1 October 2024;
- 201 Guntawong Road, Tallawong Civil Engineering Report by Indesco dated 4 March 2024;
- 201 Guntawong Road, Tallawong NSW 2762 Data Gap Assessment by JBS&G dated 19 January 2024;
- Landcom Salinity Assessment and Management Plan by JBS&G dated 7 November 2022;
- Flood Impact and Risk Assessment by TTW dated 10 January 2025;
- Architectural Drawings by DJRD Architects;
- Landscape Drawings by Site Image.

2.0 Site Description

The site is known as 201 Guntawong Road, Tallawong, NSW, 2762 (the site), and is legally described as part of Lot 1 in Deposited Plan 1283186. The site is located at the corner of Guntawong Road and Clarke Street, Tallawong and is approximately 4 hectares in area. The site has an approximately 100-metre-long frontage to Guntawong Road along its northern boundary. Nirmal Street provides a partial frontage along the eastern boundary of the site with plans to extend Nirmal Street to provide a future connection to Guntawong Road.

The site is predominantly cleared land and consists of grassland with several patches of remnant native vegetation particularly within the northern portion of the site. As a result of precinct wide rezonings, the surrounding locality is currently transitioning from a semi-rural residential area to a highly urbanised area with new low to medium density residential development with supporting services. The site is located approximately 1.5km to the northwest of Tallawong Metro Station and is also serviced by an existing bus stop along Guntawong Road.

Figure 1 below provides an aerial image of the site.



Figure 1: Aerial Photograph of Site (Source: Urbis, 2024)

2.1 Existing Services

Taylor Thomson Whitting has conducted a Before You Dig Australia (BYDA) enquiry as part of the master plan stage to identify known in-ground public assets that may impact the STHS works.

The site survey contains location of existing easements within the site, as well as existing water, sewer, electricity, gas and communications services surrounding the site. It is recommended that services be physically located prior to detailed design to identify any clash points with proposed in-ground infrastructure. The design must be progressed in accordance with the relevant services consultants' design advice and in consultation with all relevant statutory authorities.

2.2 Geotechnical Conditions

The geotechnical investigation by PSM was reviewed in preparing this report. Five boreholes were obtained at depths ranging from 8.66m to 11.1m to assess the general condition of the site. The subsurface conditions across the site were identified to have a depth of silty gravelly clay, gravelly sand and cobbles encountered ranging from 0.1-0.2m. It is underlain by natural soil of clay to gravelly clay trace sand of medium to high plasticity, which is in turn underlain by shale and laminate bedrock at depths ranging from 1.62-9.8m.

PSM recommends that for batters up to 3m in height that are above the groundwater table, design batter slopes are as shown in **Figure 2** below, subject to the following recommendations:

- The batters are to be protected from erosion.
- Permanent batters are to be drained.
- Temporary batters shall not be left unsupported for longer than 1 month without further advice. Batters should be inspected by a geotechnical engineer following significant rain events.
- Where loads are imposed or structures/services are located within on batter height of the crest of the batter, further advice should be sought

Table 11 – Design Batter Slope Angles

Unit	Temporary	Permanent
SOIL UNITS, e.g., ENGINEEERED FILL, NATURAL SOIL	2H : 1V	2.5H : 1V
BEDROCK A TO C	1H : 1V	1.5H : 1V

Figure 2: Design Batter Slopes (Source: PSM Geotechnical Investigation Report – Table 11)

Steeper batter slopes maybe be possible subject to further advice.

Pavement design advice and California Bearing Ratios (CBR) are covered in Section 5.1 of this report.

The following will also be required to inform the Civil design:

- Ground improvement specification if required for the site pending investigation results;
- Identify any project risks and suggested mitigation measures, e.g. inground obstructions;

Water table and potential impacts. Comment on suggested waterproofing systems for basements, e.g. tanked, drained, etc, if required for the site.

3.0 **Proposed Activity Description**

The proposed activity is for the construction and operation of a new high school known as Schofields - Tallawong High School. The new high school will accommodate up to 1,000 students. The school will provide 49 permanent teaching spaces (PTS), and 3 support teaching spaces (STS) across three buildings.

The buildings will be three-storey in height and will include teaching spaces, specialist learning hubs, a library, administrative areas and a staff hub. Additional core facilities are also proposed including a standalone school hall, a carpark, a pick up and drop off zone along Nirmal Street, two sports courts and a sports field.

Specifically, the proposal involves the following:

- Three learning hubs (three-storeys in height) accommodating 49 general teaching spaces and 3 support learning units (SLUs).
- Other core facilities including amenities, library, staff hub and administrative areas.
- Standalone school hall.
- Separate carpark with 72 spaces.
- Open play space including sports courts and sports field.
- Public Domain Works.

The proposed site access arrangements are as follows:

- Main pedestrian entrance to be located off Nirmal Street.
- Kiss and drop zone proposed along Nirmal Street.
- Onsite parking access via Nirmal Street.

In addition to the above, the proposed Civil works will include the construction of the following:

- Temporary OSD basin.
- Raingarden / bioretention basin.
- Stormwater pit and pipe network.
- Bulk Earthworks.
- Batter slopes and retaining walls.

Figure 3 provides an extract of the proposed architectural site plan.



Figure 3: Proposed Site Plan (Source: DJRD Architects, 2024)

It is noted that the future road to the south of the school site (also referred to as Road 4) is not included in the proposed works. Other road improvements including Hambledon Road extension to the west of the site and the intersection of that road with Guntawong Road are to be designed and delivered by others.

4.0 Stormwater

4.1 Stormwater Quantity

4.1.1 Existing Stormwater

The proposed school site of 4 hectares is located within a greenfield site with a total area of approximately 29 hectares. The site is gently sloped and drains in a westerly direction towards First Ponds Creek. For the purposes of stormwater assessment, the existing site is considered to be 100% pervious.

There are no formal stormwater pit and pipe network identified within the site area. The site drains towards first ponds creek currently conveyed via overland flow. The survey investigation identified two existing easements to drain water to the existing creek as shown in **Figure 4** below. One is a 4.40m wide to the west adjacent Ashburton Cres (which does not impact the school site) and another one just south of Nirmal and Marchant Street intersection which permits surface discharge of upstream catchments onto the school site and overland flows to First Pond Creek to the west. This easement affecting the site is further described in section 4.1.3 of this report.



There are two aspects to the stormwater management being the internal stormwater network and provisions for the upstream catchments which is diverted outside the school facility.

4.1.2 Proposed Stormwater Management: Internal Stormwater Network

The proposed stormwater design is to be in accordance with the relevant Australian Standards, Australian Rainfall and Runoff 2019 (ARR 2019), the Council's DCP, Council's Water Sensitive Urban Design (WSUD) handbook, Council's Engineering Guide for Development (EGD) and SINSW's EFSG requirements.

The site is classified as both an R2 and R3 zone. Sites within the R2 zone require a temporary On-site Detention (OSD) and water quality treatment, while those in the R3 zone must provide a temporary OSD and permanent water quality treatment. This is because the proposed external regional stormwater basin has not yet been installed and does not therefore currently provide water quality treatment for land in the R3 zone. The proposed stormwater design complies with the R2 and R3 requirements with the provision of an OSD and water quality treatment (bioretention basin). We note that the OSD basin and treatment relating to R2 can be removed once the precinct-wide detention basin has been constructed. Stormwater treatment for the R3 zone will need to be permanently managed on site.

Council has adopted a WSUD approach to integrate water cycle management into urban planning and design and reduce the impacts of new works on natural environment and waterways. This is proposed to be implemented for the site and is discussed in Section 4.1.3 and Section 4.2.

The stormwater design can be separated into two categories; roof stormwater and surface stormwater. All roof stormwater will be collected through the use of gutters and downpipes and directed to a series of rainwater tanks. The overflow from the rainwater tanks is to be conveyed to the in-ground systems discharging to the proposed OSD basin.

In-ground pipes shall convey stormwater to cater for the minor 20% AEP and major 1% AEP storm events as per EFSG. Where pipe capacity is exceeded, overland flow paths will convey the anticipated flows in the 1% AEP storm event.

4.1.3 Proposed Stormwater Management: Upstream Catchment Overland Flow

The additional existing overland flow from upstream shown in **Figure 5** is estimated to serve a catchment of approximately 20.20 hectares, which is estimated to discharge in excess of 9,000 L/s of water in the 1% AEP storm event. The point of discharged is proposed to be moved to the south of the school site (works by others) to align with the future road along the southern boundary. To prevent overland flows from impacting the school facilities a wall will be constructed along the southern site boundary adjacent to the proposed car park and the surface levels within the car park will be raised above the PMF level of the overland flow path – this is discussed further in the Flood Impact and Risk Assessment Report prepared by TTW. The future construction of Road 4 (by others) is understood to include provision for incorporating the existing overland flows in large culverts within the road corridor which will connect to the future Hambledon Road drainage system to the southwest of the school.



Figure 5: Approximate Catchment and Flows Draining to the Site (existing condition)

The proposed school site is not affected by backwater flooding by First Ponds Creek (Riverine Medium Flood Risk Precinct) in the 1% AEP and PMF storm events. Refer to the Flood Impact and Risk Assessment prepared by TTW for further details.

4.1.4 Onsite Stormwater Detention

Council's DCP Part J: Water Sensitive Urban Design and Integrated Water Cycle Management stipulates that OSD systems are generally required for "all business and industrial development, excluding subdivisions, with a development footprint greater than 150m² and that is located within an on-site stormwater detention area as defined by Council's OSD map". As per the map, the site is identified as a land zoned under north-west growth area which has separate OSD requirements.

Figure 6 below shows that the site location means that no permanent OSD is required for the site. It is understood that temporary OSD will be required for the proposed new school site until precinct wide permanent stormwater provisions have been constructed. Council have indicated that the permanent basin is planned for construction by 2026/27.



Figure 6: Land Zoned Under North West Growth Area SEPP – OSD Requirements (Source: BCC)

The temporary OSD basin will comply with Appendix B: OSD General Guidelines and Checklist of the BCC EGD. OSD is designed in line with the Council OSD spreadsheet and subject to the following requirements outlined in clause C of Section A.3, BCC EGD and provided in **Figure 7**.

- c) The following OSD parameters and conditions apply:
 - As much as possible of the site area is to drain through the OSD system(s) with an absolute minimum of 85%.
 - ii. All systems shall use a minimum of two orifice plates to control flows. The 1.5 year orifice shall be designed to convey a maximum of 40 l/s. the 100 year orifice shall be designed to convey a maximum of 190 l/s. the minimum volume up to the 1.5 year weir level = 300m³/ha. The minimum volume up to the 100 year weir level = 455 m³/ha. For more detailed information see Council's WSUD Standard Drawing A(BS) 175M.

Figure 7: OSD Parameters and Conditions (Source: BCC EGD)

A preliminary temporary OSD volume of 1,820m³ has been calculated for the site which will be provided as an open basin. The basin will discharge to the western site boundary to match the existing situation. We

understand that the future Hambledon Road design allows for this discharge. Refer to drawing no. STHS-TTW-01-00-DR-C-04103 for details.

4.2 Stormwater Quality

Stormwater quality treatment is required to comply with the requirements outlined in Table 2 of Part J Section 4.2: Water Quality. Council requires all activity to achieve a minimum percentage reduction of the post works average annual load of pollutants shown in **Figure 8** below.

The Blacktown City Council developer handbook for Water Sensitive Urban Design (WSUD) also specifies that industrial or commercial developments with carparks or manoeuvring areas greater than 1,000m² must provide a device that specifically targets hydrocarbons from the treatment train. Council requires the post activity average annual load reduction of 90% total hydrocarbons.

Pollutant	% post works average annual load reduction
Gross pollutants	90
Total suspended solids	85
Total phosphorous	65
Total nitrogen	45
Total hydrocarbons	90

Figure 8: BCC Water Quality Targets (Source: BCC DCP 2015)

The proposed activity will ensure the implementation of appropriate water quality treatment for stormwater runoff to reduce the discharge of pollutants from paved and other impermeable surfaces into waterways and council drainage systems. Stormwater quality reduction targets and maintenance of treatment measures can also be met through a combination of the following treatment options:

- Silt arrestors and trash screens;
- Oil arrestors;
- Ocean Protect StormFilter Cartridges (or equivalent);
- Ocean Protect Oceanguard Pit Inserts (or equivalent);
- Rainwater reuse tanks;
- Swales, bioretention swales and buffer strips;
- Constructed Wetlands;
- Bioretention basins;
- Raingardens.

4.2.1 MUSIC Modelling

The proposed stormwater quality treatment train was modelled using the Model for Urban Stormwater Improvement Conceptualisation (MUSIC) to ensure pollutant load target reductions are met as required by the Blacktown City Council. The results of the modelling were compared to the reduction targets prescribed by the Blacktown City Council to determine the effectiveness of the proposed measures.

MUSIC simulates the performance of a group of stormwater management measures, configured in series or in parallel to form a "treatment train" against historic rainfall event data sets. It is the industry standard water quality modelling software developed by the MUSIC development team of the Cooperative Research Centre for Catchment Hydrology (CRCCH).

4.2.2 Proposed Treatment Train

Stormwater runoff on site is proposed to be treated by a raingarden/bioretention basin. Surface runoff from external areas will be captured and directed to the bioretention before it is detained in the temporary OSD basin. For modelling purposes, 75% of the roof catchment has been designed to be reticulated to the rainwater tanks and directed to the bioretention basin. Further, an initial volume of 10kL RWT per building has been assumed. This is to be further confirmed by the hydraulic engineer.

The MUSIC model layout is shown in Figure 9 below.



Figure 9 - Proposed Treatment Train

The details of each approximate catchment and treatment are tabulated below:

Table 1 – Catchment Details and Respective Treatments

Catchment	Area (m²)	Impervious Area (%)	Treatment
Roof (to RWT)	5,285	100	Rainwater tanks, Bioretention
Roof (Bypass)	1,765	100	Bioretention
Surface Area	37,170	50	Bioretention

The assumed impervious areas and catchment sizes are subject to adjustment following the further resolution of landscape design and the proposed treatment train updated accordingly.

4.2.3 Treatment Train Effectiveness

The results of the proposed treatment train compared to the council targets are tabulated below:

Pollutant	Min. Required Reduction (%)	Modelled Reduction (%)
Total Suspended Solids (TSS)	85	86.75
Total Phosphorus (TP)	65	68.01
Total Nitrogen (TN)	45	54.41
Gross Pollutants (GP)	90	100

Table 2 – MUSIC Model Results

As shown, the activity meets the Council requirements for pollutant reduction based on the proposed treatment train consisting of rainwater reuse, pit-insert filter baskets and filter cartridges.

4.3 Erosion and Sediment Control

An erosion and sediment control plan (ESCP) will be implemented during the construction stage to mitigate soil erosion and control the discharge of stormwater laden with sediment, nutrients and other pollutants to adjoining properties, bushland, roadways or receiving water bodies. Stormwater controls on site are detailed in ESCPs which are in accordance with Council's DCP and regulatory authority guidelines including Landcom NSW's Managing Urban Stormwater, Soils and Construction ("Blue Book").

The disturbance of the site during construction must be controlled through erosion prevention and sediment control measures. Typical provisions for a site of this type and scale would include:

- A sediment basin will be required at the lowest point of the site to capture runoff from the construction site and it is likely that this will be designed for future modification to the OSD basin.
- Silt fences to prevent silt and waste being washed into neighbouring sites and streets and may be integrated with safety fencing.
- Catch drains with hay bales to carry and treat site runoff
- Sedimentation basin(s) to be installed at the low point of site excavation.
- Shaker grids at the construction site entrance(s) to ensure that vehicles and machinery leave the site with clean wheels.
- Pits will have silt protection installed to prevent silt from entering the stormwater system during construction.

The proposed Erosion and Sediment Control Plan for the site is attached in **Appendix A**. Refer to drawing no. STHS-TTW-01-00-DR-C-02101 for details.

5.0 Civil Works

5.1 Pavement Design

The geotechnical report by PSM indicated a California Bearing Ratio (CBR) value of between 1.5% to 2.5%. The report recommended a design CBR of 2% is to be adopted for subgrade and fill formed in bulk earthworks. However, the pavement design is dependent on the material at the finished subgrade levels. It is further recommended that specific CBR testing be undertaken at subgrade level when pavement layouts are finalised and that CBR testing be undertaken for new imported material. The pavement design has adopted a minimum CBR of 3% with subgrade improvement to be provided at locations where this is not achieved.

Appropriate subgrade improvement may involve the placement of a select fill layer of good quality granular material below the pavement or lime stabilisation of the subgrade soils. This will be further updated in detailed design with the final Geotechnical recommendations.

Pavements with vehicular traffic are designed with capacity for the proposed design vehicle and vehicular movements and to cater for a minimum 25-year design life in accordance with the EFSG.

The proposed pavement plan for the site, subject to detailed design, is included in **Appendix A**. Refer to drawing nos. STHS-TTW-01-00-DR-C-7000s for details.

5.2 Earthworks

The geotechnical report and the draft bulk earthwork specification (BES) reports prepared by PSM were reviewed in preparing this report. Additionally, the salinity assessment and management plan (SAMP), detailed site investigation (DSI), data gap assessment (DGA), long term environmental management plan (LTEMP) and remediation action plan (RAP) reports prepared by JBS&G were also reviewed.

For further information, refer to the original reports by PSM and JBS&G.

Requirements for the removal of topsoil and any ground improvement will be dependent on the finalisation of geotechnical investigations of the site and the proposed finished level. It is anticipated that major earthworks will be required to accommodate the proposed school works.

5.2.1 Site Preparation

As noted in the BES report, the site preparation works will include the following:

- Removal of stockpiles and mounds.
- Clearing of the area including removal and disposal of all trees, stumps, roots, bush, other organic material, all vegetation both living and dead, all minor man-made structures (e.g. fences) and all rubbish.
- Grubbing operations shall be carried out to a minimum depth of 0.5m below the surface, where grubbing is required.
- Stripping of topsoil and stockpiling for potential blending with Engineered Fill (refer Section 3.3.2 of the BES).
- Demolition of structures as directed by the Principal. Extent of demolition works are not addressed by this Specification.
- Decommissioning the services from the pre-existing infrastructure. This is to include backfilling any voids such that they do not collapse or undergo excessive settlement under the weight of the filling and building loads. Backfilling is to be undertaken with one of the following materials:

i. Cement stabilised sand (min. 3% cement) placed in accordance with the supplier requirements or

ii. Mass concrete or grout as approved by PSM.

iii. Engineered fill placed in accordance with Clauses 3.5 and 3.6 of the of the BES.

5.2.2 Excavation Works

Site soils are considered to be sodic to highly-sodic. As such, the SAMP recommends that excavation be limited to a maximum of 1m to minimise exposure of more sodic soils. Any sodic soils would then need to either be used as fill at a suitable depth or disposed of appropriately off site. All excavated materials from the site will be reused if deemed suitable.

The DSI and DGA identified the presence of various contaminants that pose a risk to human health, including asbestos containing material (ACM) and lead. However, the risks associated with the contamination can be managed through the implementation of the remediation measures described in the RAP to make the land suitable for the proposed activity.

5.2.3 Filling Works

All engineered fill is advised to be placed on bedrock, natural insitu material of at least stiff consistency, engineered compacted fill placed in accordance with this or other approved specifications for which the Geotechnical Inspection and Testing Authority (GITA) has a Level 1 certificate certifying compliance with that approved specification and of at least stiff consistency or existing fill/ pavements/ slabs and other materials as approved by PSM.

Imported engineered fill is to conform to one of the following definitions as noted in the BES, while site won material shall comprise material won from excavations on site including natural and existing fill, including crushed concrete pavement on site. The engineered fill shall be approved by the GITA as suitable for use in a structural fill.

5.2.4 Cut and Fill Volumes

A cut and fill analysis has been conducted to determine the bulk earthworks volumes. Due to constraints where the school interfaces with proposed road levels and considerations to minimise the depth of cut, a greater amount of fill is required to achieve suitable site levels.

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I he	assessment	determined	the	tollowing	volumes.
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TOTAL	Volume (m ³)
Cut	-6,723
Fill	12,446
Balance	5,723

A bulk earthworks plan has been designed in light of the above. This will be updated further in detailed design phase. Refer to drawing no. STHS-TTW-01-00-DR-C-03101.

5.3 Retaining Walls

As advised by the geotechnical report prepared by PSM, the following factors should be considered for the appropriate retaining wall design:

- Performance,
- Ground conditions (this is addressed with the design parameters);
- Groundwater;
- Surcharge loading;
- The proximity of structures, buildings, roads, etc.

- Cost to build and maintain and other constraints such as real estate, neighbouring site/boundary, aesthetics, legislation, etc.
- Proposed wall geometry;
- Effective strength parameters when assessing the earth pressure on retaining structures;
- Lateral earth pressures of soil units, and a lateral pressure of 10 kPa for vertical cuts in the bedrock units.
- Surcharge loads behind retention;
- Water pressure (depending on the type of structure).

Refer to Figure 10 below for design batter slope angles for retaining wall.

Table 12 – Design Batter Slope Angles

Inferred Unit	Bulk Unit Weight (kN/m³)	Poisson's Ratio	Lateral Earth Pressure Coefficient at Rest K₀	Active Lateral Earth Pressure Coefficient K _a	Passive Lateral Earth Pressure Coefficient K _p	
ENGINEERED FILL / NATURAL SOIL	18	0.3	0.5	0.33	3	



For further information, refer to the original report by PSM.

5.4 Public Domain Works

The proposed public domain proposals include the following works as shown on the public domain civil drawings in **Appendix A**:

- Construction of Nirmal Street western half-road including kiss and drop and footpath along the boundary length of the Nirmal Street frontage (from Guntawong Road to the future Road 4 intersection);
- Construction of Nirmal Street Intersection. Assumed priority intersection with tie-in to existing Guntawong Road levels with allowance for the potential future upgrade to roundabout;
- Widening of Guntawong Road and provision of bus stops in dedicated bus lanes— subject to a separate approval;
- Provision of footpath on Nirmal Street;
- Installation of road drainage infrastructure in Nirmal Street;
- Signage and line marking in relation to above works;
- The public domain works on Nirmal Street will include any required relocation or augmentation of existing services (including power connection and street lighting as per the electrical design by others).

In order to reduce the risk of extensive reconstruction of roads surrounding the school in the future, the design for the above works is coordinated with the following design documentation by others:

- Hambledon Road Extension Blacktown City Council (Tender Issue, November 2022)
 - The TTW design matches the proposed levels within Guntawong Road and is coordinated with the future stormwater network.
 - The kerb line of the proposed intersection of Guntawong Road and Nirmal Street is coordinated with the roundabout intersection alignment contained within this drawing set.
- 151 and 161 Tallawong Road Bathla / Enspire (Subdivision Works cert, December 2024)

- The TTW design matches the proposed road horizontal and vertical alignment of Nirmal Street. It also adopts the amended upstream stormwater discharge point in Road 4.
- 165 Guntawong Road Metro / C&M Consulting Engineers (S138 amended September 2024)
 - The TTW design matches the proposed road horizontal and vertical alignment of Nirmal Street.
- 201 Guntawong Road Landcom / Indesco (draft DA February 2024)
 - The TTW design adopts the general principals of the proposed Landcom residential subdivision design, with the deletion of proposed internal site roads. Figure 11 shows an extract from Landcom Drawing 8553-DA-010 [B] with the school site highlighted yellow and the deleted sections of Road 6 (Blue) and Road 3 (Red) which are no longer required.



Figure 11: Design Batter Slopes (Source: PSM Geotechnical Investigation Report – Table 12)

For further information, refer to the original report by PSM.

6.0 Mitigation Measures

The following mitigation measures shown in Table 3 below are to be implemented as outlined in this report.

Mitigation Number/Name	Aspect/Section	Mitigation Measure	Reason for Mitigation Measure
Erosion and Sediment Control	Prior to commencement of any construction works and during construction works.	The proposed activity will implement measures as documented generally in accordance with NSW Department of Housing's Managing Urban Stormwater as shown in drawing no. STHS-TTW- 01-00-DR-C-02101.	To ensure protection of downstream drainage lines, assets, ecosystems, or existing hydrological systems from silt, waste and sediment from the site.
Upstream catchment overland flow	Prior to commencement of operation.	The proposed activity will include measures to divert stormwater overland flow from upstream around the school facilities. A temporary wall at the edge of the carpark will be provided until the permanent stormwater works is completed in the future Road 4.	To divert the flow path from upstream catchments away from the school facility and to ensure that the site is protected against floodwaters for all events, including the Probable Maximum Flood (PMF) event.
Stormwater Peak Flows	Prior to commencement of operation.	The proposed activity will include provision of temporary OSD, in accordance with Council's OSD spreadsheet, until the precinct-wide permanent stormwater provisions have been constructed. Site overland flow path for stormwater in excess of the pipe system capacity will also be provided.	To ensure stormwater flows for all events up to and including the 1% ARI from the high school site will have no adverse impact upon the downstream properties and existing waterbodies.
Water Sensitive Urban Design	Following the removal of temporary water quality (erosion and sediment control) measures.	The proposed activity will include provision of water quality treatment measures as part of a water-sensitive urban design as documented in this report and on the Civil Engineering drawings attached in Appendix A . Refer to Hydraulic documentation for rainwater tank sizing and reuse strategy.	To ensure the proposed activity meets Council's requirements for pollutant reduction.
Public Domain Works	Prior to commencement of operation.	 (1) Construct a zebra crossing on Guntawong Road and a wombat crossing on Nirmal Street prior to occupancy. (2) Construct a 3.5m shared path along 	To ensure that students and the local community can walk along the footpaths and cross the

Table 3 – Mitigation Measures

Mitigation Number/NameAspect/SectionMitigation MeasureReason for Measure	for Mitigation
school frontage on Nirmal Street on the school frontage side only (western) prior to occupancy. (3) Construct a 3.5m shared path along school frontage on Guntawong Road along the school frontage and on the northern side of Guntawong Road from the bus stop to the zebra crossing prior to occupancy. (4) Construct two indented bus bays on Guntawong Road able to each accommodate two buses: - Eastbound bus bay: 40 metres long - Westbound bus bay: 60 metres long In the sections of Guntawong Road comprising four lanes the cross section of Guntawong road should match with the end-state cross section of Guntawong Road where possible and appropriate. The intersection of Guntawong Road and Nirmal Street should be designed as a "Give Way' intersection with one lane on each approach. The design should provision for the future roundabout at Guntawong Road and Nirmal Street where possible and appropriate. (5) Construct Nirmal Street within the site boundary to a carriageway width of 19m from Guntawong Road along the full extent of the school frontage and dedicate it to Council prior to occupancy.	ely. re that Guntawong wide enough for bus stops and traffic. treet is an ete road, with the southbound only. With only infrastructure, uld be significant on and impacts on ad users.

Note: All mitigation measures indicated on Guntawong Road are subject to a separate approval process (not part of this REF).

7.0 Consultation

Consultation with Council has commenced in relation to the stormwater strategy and roadworks proposals for the proposed activity. A summary of these Council Consultation is provided below.

- 25 June 2024- TTW requested confirmation of stormwater requirements for the proposed activity.
- 30 July 2024- TSA meeting request with Blacktown Council.
- 08 August 2024- Meeting with Blacktown Council regarding stormwater and public domain works.
- 27 August 2024- Blacktown Council responses to queries following the meeting.
- 08 October 2024 -TTW follow-up on query.
- 11 October 2024 Blacktown Council responded noting specific requirements for land zoning within the site, OSD sizing and water quality targets.
- 18 October 2024 Pre-application meeting with Blacktown Council.
- 15 November 2024 Meeting with Blacktown Council regarding project interface.
- 29 November 2024 Follow-up meeting with Blacktown Council regarding project interface.

Refer to **Appendix B** for council correspondence regarding the stormwater requirements for the proposed activity.

8.0 Evaluation of Environmental Impacts

This report concludes that the proposed activity is suitable, will not result in unacceptable impact, and warrants approval subject to implementation of the following mitigation measures discussed in Section 6.0.

9.0 Conclusion

The site has been designed in accordance with the Blacktown Council DCP and all relevant supporting documents. The site does not require a permanent OSD system. However, temporary OSD with a minimum volume requirement of 1,820m³ will be necessary for the proposed new school site until permanent stormwater infrastructure (works by Council) is implemented across the precinct. Stormwater will be treated before it is discharged at the western boundary of the site. The proposed stormwater treatment meets the Council's pollutant reduction targets via proposed rainwater tanks and raingarden/bioretention basin. A temporary wall will be required to contain water up to the PMF event until Road 4 and associated stormwater system is constructed. Erosion and sediment control measures have been proposed for the site during construction in accordance with the NSW Department of Housing Managing Urban Stormwater ('Blue Book').

A public domain design has been completed which is coordinated with other roadworks designs by others adjacent to the site.

Based on the above, the proposed civil and stormwater works complies with the Council's DCP requirements, as well as other relevant Australian standards. We recommend the stormwater system for the proposed activity be generally in accordance with the Civil Engineering drawings attached in **Appendix A**.

The findings in this statement are based on current available information, regulations and correspondence undertaken at the time of writing.

Prepared by TTW (NSW) PTY LTD

JEMA LOPEZ Engineer

Authorised By TTW (NSW) PTY LTD

COLIN ROPE Associate Director

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Appendix A

Civil Engineering Drawings

NEW HIGH SCHOOL FOR SCHOFIELDS AND TALLAWONG SCHOFIELDS, NSW 2762



3	SCHEMATIC DESIGN FOR REF	JL	ES 1	10.01.2025							
2	SCHEMATIC DESIGN FOR REF	JL	ES 0	06.12.2024							
1	FINAL DRAFT ISSUE FOR REF	JL	ES 2	21.11.2024							
Rev	Description	Eng [Draft	Date	Rev	Description	Eng Draft	Date	Rev Description	Eng Draft	Date

NOT FOR CONSTRUCTION

NUMBER

GENERAL-00000

STHS-TTW-01-00-DR-C-00001

STHS-TTW-01-00-DR-STHS-TTW-01-00-DR-STHS-TTW-01-00-DR-

ROADWORKS-01000

STHS-TTW-01-STHS-TTW-01-STHS-TTW-01-0

STHS-TTW-01-STHS-TTW-01-0

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EROSION AND SEDIMENT CONTROL NOTES AND LEGEND SHEET 1 EROSION AND SEDIMENT CONTROL PLAN STORMWATER NOTES AND LEGEND SHEET 1 STORMWATER AND SUBSOIL DRAINAGE PLAN SHEET 1 STORMWATER AND SUBSOIL DRAINAGE PLAN SHEET 2 STORMWATER AND SUBSOIL DRAINAGE PLAN SHEET 3 STORMWATER AND SUBSOIL DRAINAGE PLAN SHEET 4 RETAINING WALL PLAN SHEET 1 **RETAINING WALL PLAN SHEET 2** PAVEMENT NOTES AND LEGEND PAVEMENT PLAN **PAVEMENT DETAILS SHEET 1 PAVEMENT DETAILS SHEET 2 PAVEMENT DETAILS SHEET 3 PAVEMENT DETAILS SHEET 4** PAVEMENT DETAILS SHEET 5

STHS-TTW-01-00-DR-C-02001 STHS-TTW-01-00-DR-C-02101 EARTHWORKS-03000 STHS-TTW-01-00-DR-C-04001 STHS-TTW-01-00-DR-C-04101 STHS-TTW-01-00-DR-C-06101 PAVEMENT-07000 STHS-TTW-01-00-DR-C-07502 STHS-TTW-01-00-DR-C-07503 STHS-TTW-01-00-DR-C-07504 STHS-TTW-01-00-DR-C-07505

STHS-TTW-01-00-DR-C-03101 EARTHWORKS CUT AND FILL VOLUMES PLAN STORMWATER-04000 STHS-TTW-01-00-DR-C-04102 STHS-TTW-01-00-DR-C-04103 STHS-TTW-01-00-DR-C-04104 STHS-TTW-01-00-DR-C-04501 STORMWATER DETAILS **RETAINING WALLS-6000** STHS-TTW-01-00-DR-C-06102 STHS-TTW-01-00-DR-C-07001 STHS-TTW-01-00-DR-C-07101 STHS-TTW-01-00-DR-C-07501

STHS-TTW-01-00-DR-C-07506 PAVEMENT DETAILS SHEET 6

STHS-TTW-01-0 STHS-TTW-01-0 STHS-TTW-01-0

NOTE: PUBLIC DOMAIN DRAWINGS LISTED IN RED







NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG



This drawing is copyright and is the property of TTW and must not be used without authorisation. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT NOTES AND LEGENDS

DRAWING TITLE

GENERAL COVER SHEET GENERAL NOTES AND LEGEND SHEET 1 STHS-TTW-01-00-DR-C-00003 GENERAL KEY PLAN STHS-TTW-01-00-DR-C-00101

STHS-TTW-01-00-DR-C-00401	GENERAL ARRANGEMENT PLAN SHEET 1
STHS-TTW-01-00-DR-C-00402	GENERAL ARRANGEMENT PLAN SHEET 2
STHS-TTW-01-00-DR-C-00403	GENERAL ARRANGEMENT PLAN SHEET 3
STHS-TTW-01-00-DR-C-00404	GENERAL ARRANGEMENT PLAN SHEET 4

00-DR-C-01001	GUNTAWONG ROAD SITEWORKS AND STORMWATER PLAN
00-DR-C-01002	NIRMAL STREET SITEWORKS AND STORMWATER PLAN SHEET
00-DR-C-01003	NIRMAL STREET SITEWORKS AND STORMWATER PLAN SHEET
00-DR-C-01201	ROAD LONGITUDINAL SECTION - GUNTAWONG ROAD
00-DR-C-01202	ROAD LONGITUDINAL SECTION - NIRMAL STREET
00-DR-C-01301	ROAD CROSS SECTIONS - GUNTAWONG ROAD
00-DR-C-01302	ROAD CROSS SECTIONS - NIRMAL STREET SHEET 1
00-DR-C-01303	ROAD CROSS SECTIONS - NIRMAL STREET SHEET 2
00-DR-C-01401	ROAD TYPICAL SECTIONS SHEET 1
00-DR-C-01402	ROAD TYPICAL SECTIONS SHEET 2
00-010-0-01402	
00-DR-C-01501	SWEPT PATH PLAN

EROSION AND SEDIMENT CONTROL-02000

SIGNAGE AND LINEMARKING-08000

00-DR-C-08121	SIGNAGE AND LINEMARKING PLAN (PUBLIC DOMAIN) SHEET
00-DR-C-08122	SIGNAGE AND LINEMARKING PLAN (PUBLIC DOMAIN) SHEET 2
00-DR-C-08123	SIGNAGE AND LINEMARKING PLAN (PUBLIC DOMAIN) SHEET (

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GENERAL

- CONTRACTOR MUST VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE PRIOR TO 1
- COMMENCEMENT OF WORKS. ANY DISCREPANCIES TO BE REPORTED TO THE SUPERINTENDENT. STRIP ALL TOPSOIL FROM THE CONSTRUCTION AREA. ALL STRIPPED TOPSOIL SHALL BE DISPOSED OF OFF-SITE UNLESS DIRECTED OTHERWISE.
- MAKE SMOOTH CONNECTION WITH ALL EXISTING WORKS.
- COMPACT SUBGRADE UNDER BUILDINGS AND PAVEMENTS TO MINIMUM 98% STANDARD MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 5.1.1. COMPACTION UNDER BUILDINGS TO EXTEND 2M MINIMUM BEYOND BUILDING FOOTPRINT
- ALL WORK ON PUBLIC PROPERTY, PROPERTY WHICH IS TO BECOME PUBLIC PROPERTY, OR ANY WORK WHICH IS TO COME UNDER THE CONTROL OF THE STATUTORY AUTHORITY: THE CONTRACTOR IS TO ENSURE THAT THE DRAWINGS USED FOR CONSTRUCTION HAVE BEEN APPROVED BY ALL RELEVANT AUTHORITIES PRIOR TO COMMENCEMENT SITE
- ALL WORK ON PUBLIC PROPERTY, PROPERTY WHICH IS TO BECOME PUBLIC PROPERTY, OR ANY WORK WHICH IS TO COME UNDER THE CONTROL OF THE STATUTORY AUTHORITY IS TO BE CARRIED OUT IN ACCORDANCE WITH THE REQUIREMENTS OF THE RELEVANT AUTHORITY. THE CONTRACTOR SHALL OBTAIN THESE REQUIREMENTS FROM THE AUTHORITY. WHERE THE REQUIREMENTS OF THE AUTHORITY ARE DIFFERENT TO THE DRAWINGS AND SPECIFICATIONS
- THE REQUIREMENTS OF THE AUTHORITY SHALL BE APPLICABLE. 7. FOR ALL TEMPORARY BATTERS REFER TO GEOTECHNICAL RECOMMENDATIONS.

REFERENCE DRAWINGS

1. THESE DRAWINGS HAVE BEEN BASED FROM, AND TO BE READ IN CONJUNCTION WITH THE FOLLOWING CONSULTANTS DRAWINGS. ANY CONFLICT TO THE DRAWINGS MUST BE NOTIFIED IMMEDIATELY TO THE ENGINEER.

CONSULTANT	DRAWING TITLE	DRAWING NUMBER	REVISION	DATE
DJRD	ARCH	STHS-DJRD-00-00-REF- A-0251	03	22.11.2024
SDG	SURVEY		А	09.10.2024
DJRD	TREE RETENTION PLAN	STHS-DJRD-00-00-REF- A-0111	03	22.11.2024
GCC	HAMBLEDON ROAD EXTENSION - RIVERSONE ROAD & DRAINAGE DESIGN	STHS-DJRD-00-00-DR-A- 0300	P02	01.11.2024
C & M CONSULTING ENGINEERS	165 GUNTAWONG ROAD, ROUSE HILL	02706_S138-201	G	12.09.2024
ENSPIRE	151& 161 TALLAWONG ROAD, ROUSE HILL	220093-DA-C05.01	5	16.12.2022

BOUNDARIES AND EASEMENTS

- 1. THE PROPERTY BOUNDARY AND EASEMENT LOCATIONS SHOWN ON TAYLOR THOMSON WHITTING DRAWING'S HAVE BEEN BASED ON INFORMATION RECEIVED FROM : SURVEYOR
- 2. TAYLOR THOMSON WHITTING MAKES NO GUARANTEES THAT THE BOUNDARY OR EASEMENT INFORMATION SHOWN IS CORRECT, TAYLOR THOMSON WHITTING WILL ACCEPT NO LIABILITIES FOR BOUNDARY INACCURACIES. THE CONTRACTOR/BUILDER IS ADVISED TO CHECK/CONFIRM ALL BOUNDARIES IN RELATION TO ALL PROPOSED WORK PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. BOUNDARY INACCURACIES FOUND ARE TO BE REPORTED TO THE SUPERINTENDENT PRIOR TO CONSTRUCTION STARTING.

SURVEY

ORIGIN OF LEVELS:	PM 43374 RL 38.274
DATUM OF LEVELS:	AHD
COORDINATE SYSTEM:	GDA 2020
SURVEY PREPARED BY:	PROJECT SURVEYORS
SETOUT POINTS:	CONTACT SURVEYOR

1. TAYLOR THOMSON WHITTING DOES NOT GUARANTEE THAT THE SURVEY INFORMATION SHOWN ON THESE DRAWINGS IS ACCURATE AND WILL ACCEPT NO LIABILITY FOR ANY INACCURACIES IN THE SURVEY INFORMATION PROVIDED TO US FROM ANY CAUSE WHATSOEVER.

UNDERGROUND SERVICES - WARNING

- 1. THE LOCATIONS OF UNDERGROUND SERVICES SHOWN ON TAYLOR THOMSON WHITTINGS DRAWINGS HAVE BEEN PLOTTED FROM DIAGRAMS PROVIDED BY SERVICE AUTHORITIES. THIS INFORMATION HAS BEEN PREPARED SOLELY FOR THE AUTHORITIES OWN USE AND MAY NOT NECESSARILY BE UPDATED OR ACCURATE.
- 2. THE POSITION OF SERVICES AS RECORDED BY THE AUTHORITY AT THE TIME OF INSTALLATION MAY NOT REFLECT CHANGES IN THE PHYSICAL ENVIRONMENT SUBSEQUENT TO INSTALLATION.
- THE CONTRACTOR MUST CONFIRM THE EXACT LOCATION AND EXTENT OF SERVICES PRIOR TO CONSTRUCTION AND NOTIFY ANY CONFLICT WITH THE DRAWINGS IMMEDIATELY TO THE ENGINEER/SUPERINTENDENT.
- 4. THE CONTRACTOR IS TO GET APPROVAL FROM THE RELEVANT STATE SURVEY DEPARTMENT, TO REMOVE/ADJUST ANY SURVEY MARK. THIS INCLUDES BUT IS NOT LIMITED TO; STATE SURVEY MARKS (SSM), PERMANENT MARKS (PM), CADASTRAL REFERENCE MARKS OR ANY OTHER SURVEY MARK WHICH IS TO BE REMOVED OR ADJUSTED IN ANY WAY.
- 5. TAYLOR THOMSON WHITTING PLANS DO NOT INDICATE THE PRESENCE OF ANY SURVEY MARK. THE CONTRACTOR IS TO UNDERTAKE THEIR OWN SEARCH.

BEFORE YOU DIG AUSTRALIA (BYDA)

- 1. PUBLIC SERVICE UTILITY INFORMATION SHOWN ON PLAN HAS BEEN COMPLIED FROM INFORMATION RECEIVED FROM DIAL BEFORE YOU DIG INQUIRY, REFERENCE NUMBER No. 37862101 OBTAINED ON 21.10.2024 UNLESS SPECIFICALLY SHOWN OTHERWISE, THIS LOCATION AND DEPTH OF SERVICES SHOWN ON THIS PLAN HAVE NOT BEEN VERIFIED.
- 2. THE LOCATION OF SERVICES SHOWN ON THIS DRAWING HAVE BEEN PLOTTED AS ACCURATELY AS POSSIBLE FROM DIAGRAMS PROVIDED BY SERVICE AUTHORITIES AND SHOULD BE CONFIRMED BY SITE INSPECTION."

SITE WORKS

- 1. ALL BASECOURSE MATERIAL TO COMPLY WITH RMS SPECIFICATION NO 3051 AND COMPACTED TO
- MINIMUM 98% MODIFIED MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 5.2.1. 2. ALL TRENCH BACKFILL MATERIAL SHALL BE COMPACTED TO THE SAME DENSITY AS THE ADJACENT MATERIAL
- 3. ALL SERVICE TRENCHES UNDER VEHICULAR PAVEMENTS SHALL BE BACKFILLED WITH AN APPROVED SELECT MATERIAL AND COMPACTED TO A MINIMUM 98% MODIFIED MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 5.2.1

PUBLIC DOMAIN WORKS

APPROVED.

TENDER DOCUMENTATION

- STRUCTURAL/CIVIL WORKS DRAWINGS
- SUPERINTENDENT
- REQUIREMENTS. ETC.

DESIGN AND CONSTRUCT DOCUMENTATION

- FULLY WITH OTHER CONSULTANTS. FUNCTIONAL OBJECTIVE OF THIS DOCUMENTATION.

SAFETY IN DESIGN

CONTRACTOR TO REFER TO APPENDIX B OF THE CIVIL SPECIFICATION FOR THE CIVIL RISK AND SOLUTIONS REGISTER.

- 1. EXISTING SERVICES RELOCATE.
- 2. EXISTING STRUCTURES FAR AS PRACTICABLY POSSIBLE FROM EXISTING STRUCTURE(S).
- 3. EXISTING TREES
- 4. GROUNDWATER
- 5. EXCAVATIONS ENGINEER.
- 6. GROUND CONDITIONS REPORT BY
- PSM
- INVESTIGATION (REF PSM4693-012L) DATED 21 OCTOBER 2024) JBS&G
- 162496 DATED 1 OCTOBER

7. HAZARDOUS MATERIALS EXISTING ASBESTOS PRODUCTS & CONTAMINATED MATERIAL MAY BE PRESENT ON SITE. CONTRACTOR TO ENSURE ALL HAZARDOUS MATERIALS ARE IDENTIFIED PRIOR TO COMMENCING WORKS. SAFE WORKING PRACTICES AS PER RELEVANT AUTHORITY TO BE ADOPTED AND APPROPRIATE PPE TO BE USED WHEN HANDLING ALL HAZARDOUS MATERIALS. REFER TO GEOTECHNICAL/ENVIRONMENTAL REPORT BY

- JBS&G
- OCTOBER 2024
- 67774 /162926 DATED 1 OCTOBER 2024
- 8. CONFINED SPACES
- 9. MANUAL HANDLING AND ASSESSMENTS ARE IN PLACE PRIOR TO COMMENCING WORKS.
- 10. WATER POLLUTION CONSTRUCTION WORKS CONTAMINATING THE SURROUNDING ENVIRONMENT.
- 11. SITE ACCESS/EGRESS PERSONNEL AND PUBLIC.
- 12. VEHICLE MOVEMENT VEHICLE MOVEMENTS WHERE NECESSARY.

3	SCHEMATIC DESIGN FOR REF	JL ES 10.01.2025							
2	SCHEMATIC DESIGN FOR REF	JL ES 06.12.2024							
1	FINAL DRAFT ISSUE FOR REF	JL ES 21.11.2024							
Rev	Description	Eng Draft Date	Rev Description	Eng Draft	Date	Rev	Description	Eng Draft	Date



1. PUBLIC DOMAIN WORKS ARE NOT TO COMMENCE UNTIL THESE DRAWINGS ARE STAMPED AS

1. THESE DRAWINGS ARE PRELIMINARY DRAWINGS ISSUED FOR TENDER AS AN INDICATION OF THE EXTENT OF WORKS ONLY. THEY ARE NOT A COMPLETE CONSTRUCTION SET OF DRAWINGS. 2. TO DETERMINE THE FULL EXTENT OF WORK, THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS AND OTHER CONTRACT DOCUMENTS. ALLOW FOR ALL ITEMS SHOWN ON ARCHITECTURAL AND OTHER DRAWINGS AS NOT ALL ITEMS ARE SHOWN ON THE

3. SHOULD ANY AMBIGUITY, ERROR, OMISSIONS, DISCREPANCY, INCONSISTENCY OR OTHER FAULT EXIST OR SEEM TO EXIST IN THE DOCUMENTS, IMMEDIATELY NOTIFY IN WRITING TO THE

4. RATES SHOWN ON THE DRAWINGS ARE FOR THE FINAL STRUCTURE/CIVIL WORKS IN PLACE AND DO NOT ALLOW FOR ANY WASTAGE, ROLLING MARGINS, OVER SUPPLY OR FABRICATION

1. THE LEVEL OF DETAIL / DESIGN REFLECTED IN THESE DOCUMENTS IS BASED ON THE UNDERSTANDING THIS WILL BE BUILT AS PART OF A DESIGN & CONSTRUCT CONTRACT. 2. THE CONTRACTOR SHALL RETAIN THE RESPONSIBILITY TO UNDERTAKE DETAILED DESIGN, CONFIRM COMPLIANCE WITH RELEVANT STANDARDS, CONSENT CONDITIONS & THE SPECIFICATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THE FINAL DESIGN IS CO-ORDINATED

4. NO VARIATION WILL BE ACCEPTED FOR DESIGN AMENDMENTS REQUIRED TO MEET THE

CONTRACTOR TO BE AWARE EXISTING SERVICES ARE LOCATED WITHIN THE SITE. LOCATION OF ALL SERVICES TO BE VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCING WORKS. CONTRACTOR TO CONFIRM WITH RELEVANT AUTHORITY REGARDING MEASURES TO BE TAKEN TO ENSURE SERVICES ARE PROTECTED OR PROCEDURES ARE IN PLACE TO DEMOLISH AND/OR

CONTRACTOR TO BE AWARE EXISTING STRUCTURES MAY EXIST WITHIN THE SITE. TO PREVENT DAMAGE TO EXISTING STRUCTURE(S) AND/OR PERSONNEL, SITE WORKS TO BE CARRIED OUT AS

CONTRACTOR TO BE AWARE EXISTING TREES EXIST WITHIN THE SITE WHICH NEED TO BE PROTECTED. TO PREVENT DAMAGE TO TREES AND/OR PERSONNEL, SITE WORKS TO BE CARRIED OUT AS FAR AS PRACTICABLY POSSIBLE FROM EXISTING TREES. ADVICE NEEDS TO BE SOUGHT FROM ARBORIST AND/OR LANDSCAPE ARCHITECT ON MEASURES REQUIRED TO PROTECT TREES.

CONTRACTOR TO BE AWARE GROUND WATER LEVELS ARE CLOSE TO EXISTING SURFACE LEVEL TEMPORARY DE-WATERING MAY BE REQUIRED DURING CONSTRUCTION WORKS.

DEEP EXCAVATIONS DUE TO STORMWATER DRAINAGE WORKS IS REQUIRED. CONTRACTOR TO ENSURE SAFE WORKING PROCEDURES ARE IN PLACE FOR WORKS. ALL EXCAVATIONS TO BE FENCED OFF AND BATTERS ADEQUATELY SUPPORTED TO APPROVAL OF GEOTECHNICAL

CONTRACTOR TO BE AWARE OF THE SITE GEOTECHNICAL CONDITIONS. REFER TO GEOTECHNICAL

SCHOFIELDS TALLAWONG HIGH SCHOOL SITE 1 GUNTAWING ROAD GEOTECHNICAL

- DETAILED SITE INVESTIGATION REPORT, PROPOSED TALLAWONG HIGH SCHOOL (REF 67774/

- REMEDIAL ACTION PLAN, PROPOSED TALLAWONG HIGH SCHOOL (REF 67774/162922) DATED 1

- LONG TERM ENVIRONMENTAL MANAGEMENT PLAN, PROPOSED TALLAWONG HIGH SCHOOL (REF

CONTRACTOR TO BE AWARE OF POTENTIAL HAZARDS DUE TO WORKING IN CONFINED SPACES SUCH AS STORMWATER PITS, TRENCHES AND/OR TANKS. CONTRACTOR TO PROVIDE SAFE WORKING METHODS AND USE APPROPRIATE PPE WHEN ENTERING CONFINED SPACES.

CONTRACTOR TO BE AWARE MANUAL HANDLING MAY BE REQUIRED DURING CONSTRUCTION. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ENSURE MANUAL HANDLING PROCEDURES

CONTRACTOR TO ENSURE APPROPRIATE MEASURES ARE TAKEN TO PREVENT POLLUTANTS FROM

CONTRACTOR TO BE AWARE SITE WORKS OCCUR IN CLOSE PROXIMITY TO FOOTPATHS AND ROADWAYS. CONTRACTOR TO ERECT APPROPRIATE BARRIERS AND SIGNAGE TO PROTECT SITE

CONTRACTOR TO SUPPLY AND COMPLY WITH TRAFFIC MANAGEMENT PLAN AND PROVIDE ADEQUATE SITE TRAFFIC CONTROL INCLUDING A CERTIFIED TRAFFIC MARSHALL TO SUPERVISE

CIVIL INSPECTION CERTIFICATES

THE FOLLOWING MUST BE PROVIDED BY THE CONTRACTOR A MINIMUM 2 WEEKS PRIOR TO THE REQUESTED DATE OF A CIVIL INSPECTION CERTIFICATE FOR OCCUPATION CERTIFICATE. SUBMISSIONS MUST BE PROVIDED PROGRESSIVELY AS WORKS ARE COMPLETED IN ACCORDANCE WITH THE CIVIL SPECIFICATION. THE PROGRAM MUST ALLOW ADEQUATE TIME FOR DEFECTS TO BE RECTIFIED SHOULD THIS BE REQUIRED.

- 1. NOTIFICATION THAT ALL CIVIL WORKS TO BE CERTIFIED HAVE BEEN COMPLETED TO ALLOW A FINAL INSPECTION TO BE UNDERTAKEN.
- 2. 2.WRITTEN CONFIRMATION FROM THE CONTRACTOR THAT ALL CIVIL SITE INSPECTION REPORTS HAVE BEEN CLOSED OUT 3. 3.CCTV (INCLUDING WINCAN LOG OR EQUIVALENT) OF ALL CIVIL STORMWATER WORKS TO BE
- CERTIFIED. 4. 4.WAE FROM A REGISTERED SURVEYOR (PDF & DWG) FOR ALL CIVIL STORMWATER TO BE
- CERTIFIED. 5. 5.WAE FROM A REGISTERED SURVEYOR (PDF, DWG & 3D TIN) FOR ALL EXTERNAL CIVIL LEVELS TO
- BE CERTIFIED. 6.HEAD CONTRACTORS STATEMENT OF CONSTRUCTION COMPLIANCE.
- 7.HYDRAULIC CONTRACTORS INSTALLATION CERTIFICATE.
- 8.3RD PARTY INSTALLATION CERTIFICATES FOR PROPRIETARY PRODUCTS AND/OR D&C ELEMENTS. 9.COMPACTION TEST RESULTS IN ACCORDANCE WITH THE CIVIL SPECIFICATION. 10. 10.MATERIALS CERTIFICATES PRIOR TO INSTALLATION IN ACCORDANCE WITH THE CIVIL
- SPECIFICATION. 11. 11.WRITTEN CONFIRMATION FROM TFNSW AND/OR COUNCIL CONFIRMING COMPLETION AND ACCEPTANCE OF S138 WORKS IF APPLICABLE.

BOUNDARIES

REMOVED

_ - - - _ - - - _ - - - _ - - _ - - _ - - _ - - _ -

EXISTING

BUILDINGS

EXISTING

REMOVED



LANDSCAPE

EXISTING TREES TO BE REMOVED

EXISTING TREE TO BE RETAINED

EXISTING SERVICES



CLASSIFICATION OF EXISTING UTILITY INFORMATION

- SIGHTED, MUST BE LOCATED, THEN POTHOLED. UTILITY MUST BE PHYSICALLY SIGHTED AND MEASURED.
- ELECTRONICALLY DETECTED AND LOCATED ON SITE USING VARIOUS TRACING METHODS
- ALIGNED FROM SURFACE FEATURES AND DIGITISED DATA.
- DIGITISED DATA (DIAL BEFORE YOU DIG). D

NOTE

- 1. BELOW GROUND SERVICES CAN BE REPRESENTED AS GREY FOR EXISTING AND BLACK FOR PROPOSED DEPENDING ON THE PLAN.
- 2. EXISTING SERVICES PITS, STRUCTURES AND COLUMNS ARE ILLUSTRATED AS PER THE ORIGINAL SURVEY.



School Infrastructure NSW





NOT FOR CONSTRUCTION

PROPOSED _____ **BLOCK BOUNDARY**

PROPOSED **BUILDING ENVELOPE** FUTURE BUILDING ENVELOPE



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NTS	ES	AW	CR	
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NIRMAL STREET

NEW BUILDING **BUILDING B NEW BUILDING** FFL 44.100 BUILDING B FFL 44.100 AMAK M M |**∩**/ ADJOINS SHEE SPORTS FIELD Scale at A1 Drawn Designed ES 1:250 CR AW ARRANGEMENT PLAN Project No Originator Type Role Sheet No. Rev STHS-TTW-01-00-DR-C-00401-3

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IGEMENT PLAN	Project No	Originator	Туре	Role Sheet No.	Rev			
4	STHS	-TTW-01	-00-DR-	·C-00404	1-4			
	22.01.2025 11:51 AM							

The Jemena high-pressure gas pipeline appears to be situated far from the Endeavour trenching allocation. As long as the gap between the gas mains and Endeavour cables is at least 500mm, this should be safe. At this distance, fault currents from the Endeavour cables will not be transmitted to the gas pipe, ensuring safety. The final placement of the streetlight columns will be determined during the design stage. The council has provided the categories for both Guntawong Road has been categorized as a V category for main roads and vehicles, meaning the columns will be installed from the back of the kerb, with the exact distance to be confirmed once the lighting design is finalised. Nirimal Street, categorized as a P category for pedestrians, will have columns for both streets will be assessed according to Australian standards to ensure compliance. The streetlight design The removal of HV/LV overhead mains on Guntawong Road and the installation of a PM substation are part of this project and will be designed to comply with Endeavour Energy's standards, including easement and restriction zone requirements. The undergrounding of the HV/LV USIL40.03 USIL40.60 Ø750 RCP Ø750 RCP 22.50m@2.00% 26.42m@2.04% DSIL39.58 DSIL40.06 DEEP TRENCHING DUE TO FUTURE WATER MAIN **DIVERSION WORKS** BY OTHERS 3.8 WIDE PEDESTRIAN CROSSING 3.6m WIDE MIN. 3.7m WIDE ROADWAY PEDESTRIAN PATH PAVEMENT WIDENING TO RETAIN EXISTING DRIVEWAY 2 x 3.5m WIDE 3.5m WIDE BUS STOP TRAFFIC LANES ISLAND FOR BOTH INTERIM AND FUTURE SCENARIO 3 01502 40.0m BUS BAY 11.1m 29.0m 60.0m BUS BAY EXISTING OVERHEAD HV TO BE RELOCATED UNDERGROUND - REFER TO SERVICES NOTES USIL41.07 USIL42.80 Ø7375 RCP Ø7375 RCP 25.65m@5.74% 32.39m@5.22% DSIL39.02 DSIL41.10 INTERNAL ILP ROAD NOT REQUIRED -DELETED FROM PROPOSED WORKS RETENTION / RELOCATION OF EXISTING GAS, WATER, COMMS SERVICES TO BE ASSESSED BY ASSET OWNERS NOTE: WORKS SHOWN TO GUNTAWONG ROAD ARE SUBJECT TO SEPARATE APPROVAL (NOT UNDER THIS REF) Engineer Drawing Title: Client NEW HIGH SCHOOL FOR NSW SCHOFIELDS School Infrastructure NSW TALLAWONG

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3	SCHEMATIC DESIGN FOR REF	JL ES	6 10.01.2025							
2	SCHEMATIC DESIGN FOR REF	JL ES	6 06.12.2024							
1	FINAL DRAFT ISSUE FOR REF	JL ES	6 21.11.2024							
Rev	Description	Eng Dra	ift Date	Rev	Description	Eng Draft	Date	Rev Description	Eng Draft	Date

Drawing	Title:	Scale at A1	Drawn	Designed	Approved	
NIF	RMAL STREET	1:250	ES	AW	CR	
RC	ADWORKS AND	Project No	Originator	Туре	Role Sheet No.	Rev
ST	ORMWATER PLAN	STHS	-TTW-01-	-00-DR-	C-01003	3-3
SH	EE12	10.01.20	25 3:15 PM			

NOTE: WORKS SHOWN TO GUNTAWONG ROAD ARE SUBJECT TO SEPARATE APPROVAL (NOT UNDER THIS REF)

3 SCHEMATIC DESIGN FOR REF JL ES 10.01.2025

2 SCHEMATIC DESIGN FOR REF JL ES 06.12.2024 1 FINAL DRAFT ISSUE FOR REF JL ES 21.11.2024 Rev Description

Eng Draft Date Rev Description

Eng Draft Date Rev Description

Eng Draft Date

Engineer:

NOT FOR CONSTRUCTION

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	IP CH 0.007	RL 46.813	IP CH 10.000	, RL 46.908	2.50%	J IP CH 20.000	RL 46.658	3.70%	, J IP CH 30.000	(5) (5)	/ % IP CH 40.002	/ , RL 45.660	χώ IP CH 42.554	o BL 45.497
IMENT		R=40.00 L=11.44	00 19	\wedge	R=15 L=11	.000 .825			_	R=60.000 L=19.280				
AHD)	46.811		46.911	46.985		46.659	46.371		46.318			45.660	45.582	
mAHD)	45.464		45.791	45.870		45.979	46.027		45.872			45.560	45.497	
	1.346		1.120	1.115		0.680	0.345		0.445			0.100	0.084	
	0.000		10.000	11.449		20.000	23.274		30.000			40.000	42.554	

KERB RETURN INVERT OF KERB LONGITUDINAL SECTION

	Scale at A1	Drawn	Designed	Approved				
		ES	AW	CR				
JN	Project No	Originator	Туре	Role Sheet No.	Rev			
WONG ROAD	STHS-TTW-01-00-DR-C-01201-3							

	, IP CH 0.000	56 RL 47.328	2000 IP CH 8.06	$\begin{bmatrix} 1 & 1 \\ 1 & 1 \end{bmatrix} \begin{bmatrix} 1 & 1 \\ 1 & 1 \end{bmatrix} \begin{bmatrix} 1 & 1 \\ 2 & 1 \end{bmatrix} \begin{bmatrix} 1 & 1 \\ $		ERIM IN STING I	ITERSECT LEVEL	ION TIES	IN TO		-1.00%				
DATUM R.L. 35.00															
VERTICAL GEOMETRY	-1	3.06 1.46%	L: 10 % G: 0.	0.00 46%	, /						99.82 -1.00%				
DESIGN SURFACE (mAHD)	47.328	47.284	47.216	47.194	47.161	47.091	46.991	46.891	46.791	46.691	46.591	46.491	46.391	46.291	
EXISTING SURFACE (mAHD)	47.328	47.252	47.021	46.832	46.623	46.249	45.922	45.881	46.040	46.305	46.467	46.438	46.318	46.187	
CUT/FILL DEPTH (m)	0.000	0.032	0.196	0.361	0.538	0.842	1.070	1.010	0.751	0.387	0.124	0.053	0.073	0.105	
CHAINAGE (m)	0.000	3.063	8.063	10.000	13.063	20.000	30.000	40.000	50.000	60.000	70.000	80.000	000.06	100.00	

NIRMAL STREET CENTRE LINE LONGITUDINAL SECTION

SCALES: HORIZONTAL 1:500 VERTICAL 1:100

	0.0	10.0	20.0	30.0	0	40.0m							
	1:500 A1				1:100	0 A3							
	0.0	2.0	4.0	6.0)	8.0m							
	1:100 A1				1:20	0 A3							
											1		
3	SCHEMA	TIC DESIG	N FOR REF	JL	ES 1	0.01.2025							
2	SCHEMA	TIC DESIG	N FOR REF	JL	ES C)6.12.2024							
1	FINAL DR	AFT ISSUE	FOR REF	JL	ES 2	21.11.2024							
Rev	Descriptio	n		Eng	Draft	Date	Rev	Description	 Eng Draft	Date	Rev Description	Eng Draft	Date

School Infrastructure NSW

Engineer:

NOT FOR CONSTRUCTION

	Scale at A1	Drawn	Designed	Approved					
		ES	AW	CR					
DN	Project No	Originator	Туре Г	Role Sheet No.	Rev				
STREET	STHS-TTW-01-00-DR-C-01202-3								
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Rev Description

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	OFFSET (m)		-10.467	-7.000	0.000	6.550	7.000	11.300	
	<u>CH 80.00</u>								
0.0	4.0 8.0 A1	12.0 16.0m 1:400 A3							
1:200 /) A1	1:400 A3							

CH 60.00

DATUM R.L. 42.00

_ __ __ __ _

DESIGN SURFACE (mAHD)

EXISTING SURFACE (mAHD)

LEVEL DIFFERENCE (m)

3 SCHEMATIC DESIGN FOR REF JL ES 10.01.2025

2 SCHEMATIC DESIGN FOR REF JL ES 06.12.2024 1 FINAL DRAFT ISSUE FOR REF JL ES 21.11.2024

		HALF ROAD TO MATCH BCC DESIGN LEVELS										
		-3.3%				-3.0%		4.0%	-2	20.7%	, D	
DATUM R.L. 40.00	_											
DESIGN SURFACE (mAHD)		41.521	41.603	41.622	41.661	41.464	41.437	41.795	41.771	41.391	41.201	41.200
EXISTING SURFACE (mAHD)	90 7 7	41.521	41.574	41.592	41.613	41.374	41.362	41.263	41.240	41.157	41.157	41.157
LEVEL DIFFERENCE (m)	0.005	000.0	0.029	0.030	0.048	0.090	0.076	0.532	0.531	0.234	0.044	0.044
OFFSET (m)		-4.041	-1.773	-1.187	0.000	6.550	7.000	10.500	11.300	13.138	13.988	13.991

4.0%

5

.578

42.

СН	40.00	

DATUM R.L. 40.00		-2.7%-2	2.9%	ő - 2.0%	-2.0%-2	2.5%		-		
DESIGN SURFACE (mAHD)	40.359	40.415	40.455	40.397	40.342	40.321	40.284	40.404	40.345	40.311
EXISTING SURFACE (mAHD)	40.359	40.420	40.455	40.379	40.331	40.324	40.321	40.317	40.313	40.311/
LEVEL DIFFERENCE (m)	0.000	-0.005	0.000	0.018	0.011	-0.003	-0.036	0.087	0.033	0.000
OFFSET (m)	-3.485	-1.379	0.000	2.832	5.615	6.440	6.891	7.417	7.937	8.163

С	\vdash	2	0	0	0

	- EXISTING F TO BE RET	OAD SURFACE AINED
DATUM R.L. 38.00		
DESIGN SURFACE (mAHD)		
EXISTING SURFACE (mAHD)		
EVEL DIFFERENCE (m)		
DFFSET (m)		

┌─ FUTURE HAMBLEDON ROAD INTERSECTION DESIGN LEVEL

		LEVELS TO MATCH BCC DESIGN									\int	- SHELEV
		4.0%		-3.0%	-3.0%			4.0%				
DATUM R.L. 42.00												
DESIGN SURFACE (mAHD)	44.018	43.662	43.689	43.885		43.689	43.662	44.019	43.995	43.884	43.878	
EXISTING SURFACE (mAHD)				43.862		43.629	43.605	43.413	43.380	43.359	43.358	
LEVEL DIFFERENCE (m)				0.023		0.060	0.057	0.606	0.616	0.525	0.520	
OFFSET (m)	-10.461	-7.000	-6.550	0.000		6.550	7.000	10.500	11.300	11.806	11.832	
CH 100 00												

				LEVELS TO MAT		_ EN	TRY TO SO	CHOOL			
DATUM R.L. 44.00		4.0%	•	-3.0%	-3.0%	TF	4.0%				
DESIGN SURFACE (mAHD)	45.107	44.747	44.774	44.967	44.771	44.744	45.101	45.097			
EXISTING SURFACE (mAHD)				44.962	44.686	44.660 /	44.456	44.448			
LEVEL DIFFERENCE (m)				0.005	0.085	0.084	0.646	0.650			
OFFSET (m)	-10.454	-6.890	-6.440	0.000	6.550	7.000	10.500	10.633			
										-	

			-3.0%	-3.0%		5.3%		2	4.9%
DATUM R.L. 44.00									
DESIGN SURFACE (mAHD)	45.905	45.899	46.049	45.839	45.963	46.151	46.149	46.117	45.096
EXISTING SURFACE (mAHD)		45.915	46.038	45.677	45.639	45.411	45.392	45.364	45.054
LEVEL DIFFERENCE (m)		-0.016	0.011	0.162	0.324	0.740	0.757	0.753	0.042
OFFSET (m)	-5.056	-4.998	0.000	7.000	7.554	10.918	11.446	11.906	16.000

CH 140.00

4.0% -3.0%

42.935 42.911 42.525

42.353 42.314 42.234

0.582 0.597 0.291

42.605

42.519 42.501

0.086 0.077

┌─ SITE LEVEL

LEVEL _____ OFFSE

CH 160.00

Eng Draft Date Rev Description

LEVELS TO MATCH BCC DESIGN

-3.0%

-3.0%

Eng Draft Date

SITE LEVEL

DATUM R.L. 46.00

DESIGN SURFACE (mAHD) _____

EXISTING SURFACE (mAHD) _ __ __ __ __ __

LEVEL DIFFERENCE (m)

OFFSET (m)

CH 180.00

CH 120.00

				1				NIF	RMAI	L STREET INTERSECTIC	N
M R.L. 46.00		1	-3.0%	-3.0%	-2.7%		-(0.9%	, D		
N SURFACE (mAHD)	46.830	46.825	46.981	46.772	46.716	46.693	46.682	46.671	46.669		
NG SURFACE (mAHD)		46.876	46.996	46.629	46.436	46.340	46.219	46.132	46.062		
DIFFERENCE (m)		-0.051	-0.015	0.143	0.280	0.353	0.463	0.539	0.607		
ET (m)	-5.350	-5.228 /	0.000	6.971	9.024	10.150	11.641	12.825	13.806		
DIFFERENCE (mAHD)	-5.350	-5.228 / -0.051 46.8	0.000 -0.015 46.5	6.971 0.143 46.6	9.024 0.280 46.4	10.150 0.353 46.3	11.641 0.463 46.2	12.825 0.539 46.1	13.806 0.607 46.0		

Engineer:

Client:

School Infrastructure NSW

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NOTE: DESIGN LEVELS CH 41.41 TO CH 116.46 ARE BASED ON GUNTAWONG ROAD DESIGN PROVIDED BY BLACKTOWN CITY COUNCIL.

SECTIONS
WONG ROAD

Scale at A1	Drawn	Designed	Approved				
1:200	ES	AW	CR				
Project No	Originator	Туре	Role Sheet No.	Rev			
STHS-TTW-01-00-DR-C-01301-3							

10.01.2025 3:16 PM

	SCHEMATIC DESIGN FOR REF	JL	ES 10).01.2025									
	SCHEMATIC DESIGN FOR REF	JL	ES 06	6.12.2024									
	FINAL DRAFT ISSUE FOR REF	JL	ES 21	.11.2024									
v	Description	Eng	Draft	Date	Rev	Description	Eng l	Draft	Date	Rev	Description	Eng Draft	Date

		-3.0%			4.0% -3	3.0%	% 25.0%	
DATUM R.L. 44.00						_		
DESIGN SURFACE (mAHD)	46.691	46.540	46.513	46.738	46.910	46.886	45.847	
EXISTING SURFACE (mAHD)	46.305	45.962	45.931	45.918	45.621	45.566	45.280	
LEVEL DIFFERENCE (m)	0.387	0.578	0.582	0.820	1.289	1.320	0.567	
OFFSET (m)	0.000	5.050	5.500	5.692	10.000	10.800	14.957	

	40.00
<u>CH</u>	40.00

		-3.0%			4.0% -	3.0%	% 25.0%	
DATUM R.L. 44.00								
DESIGN SURFACE (mAHD)	46.891	46.740	46.713	46.938 /	47.110	47.086	45.188	
EXISTING SURFACE (mAHD)	45.881	45.471	45.437	45.423	45.145	45.110	44.616	
LEVEL DIFFERENCE (m)	1.010	1.269	1.275	1.515	1.965	1.976	0.572	
OFFSET (m)	0.000	5.050	5.500	5.692	10.000	10.800	18.393	

СН	20.00

	│ WORKS (│ (TYP)	BY OTHERS								
———————		-3.0%		-11	4.0% -	3.0%	-25.0%			
DATUM R.L. 44.00						-				
DESIGN SURFACE (mAHD)	47.091	100 U	46.864	47.089 /	47.228	47.204	44.934	44.907	44.904	
EXISTING SURFACE (mAHD)	46.249	1 011	45.816	45.799	45.525	45.473	44.934	44.909	44.906	
LEVEL DIFFERENCE (m)	0.842		1.048	1.289	1.704	1.731	0.000	-0.002	-0.002	
OFFSET (m)	0.000		0.090	7.337	10.835	11.635	20.716	21.341	21.409	

<u>CH 0.00</u>

		-4.3%			-4.4%				-4.5%									
DATUM R.L. 40.00																		—
DESIGN SURFACE (mAHD)	47.328	47.230	47.147	47.057	46.974	46.889	46.824	46.796	46.702	46.621	46.582	46.523	46.493	46.411	46.331	46.269	46.204	46.170
EXISTING SURFACE (mAHD)	47.328	47.231	47.149	47.061	46.981	46.898	46.827	46.797	46.696	46.610	46.569	46.508	46.478	46.395	46.317	46.256	46.188	46.154
LEVEL DIFFERENCE (m)	0.000	-0.001	-0.003	-0.005	-0.007	-0.009	-0.003	-0.001	0.006	0.011	0.013	0.015	0.016	0.016	0.014	0.013	0.015	0.016
OFFSET (m)	0.000	2.281	4.215	6.288	8.176	10.126	11.608	12.241	14.323	16.043	16.877	18.098	18.703	20.355	21.923	23.134	24.372	25.000

		-3.0%	= T F	4.0%	
DATUM R.L. 44.00					
DESIGN SURFACE (mAHD)	46.491	46.310	46.283	46.508/ 46.670	
EXISTING SURFACE (mAHD)	46.438	46.149	46.119	46.106 45.782	
LEVEL DIFFERENCE (m)	0.053	0.161	0.164	0.402 0.887	
OFFSET (m)	0.000	6.050	6.500	6.692	

		-3.0%			4.0% -	3.0%	^{//} -25.0%	
DATUM R.L. 44.00								
DESIGN SURFACE (mAHD)	46.291	46.110	46.083	46.308	46.480	46.456	45.493	
EXISTING SURFACE (mAHD)	46.187	45.877	45.845	45.832	45.640	45.600	45.410	
LEVEL DIFFERENCE (m)	0.105	0.233	0.237	0.476	0.840	0.856	0.084	
OFFSET (m)	0.000	6.050	6.500	6.692	11.000	11.800	15.651	

	│ WORKS │ (TYP)	BY OTHERS							
		-3.0%			4.0% -3	3.0%	% 25.0%		
DATUM R.L. 44.00								L	
DESIGN SURFACE (mAHD)	46.077	45.896	45.869	46.094	46.266	46.242	44.984	44.970	
EXISTING SURFACE (mAHD)	45.996	45.502	45.464	45.448	45.087	45.048	44.984	44.983	
LEVEL DIFFERENCE (m)	0.081	0.394	0.405	0.646	1.179	1.193	0.000	-0.012	
OFFSET (m)	0.000	6.050	6.500	6.692	11.000	11.800	16.831	16.880	
CH 120.00									

		-3.0%			4.0% -3	3.0%		, o
DATUM R.L. 44.00								
DESIGN SURFACE (mAHD)	45.683	45.502	45.475	45.700 /	45.872	45.848	45.143	
EXISTING SURFACE (mAHD)	45.602	45.264	45.233	45.220	44.935	44.900	44.735	
LEVEL DIFFERENCE (m)	0.081	0.238	0.241	0.479	0.937	0.948	0.408	
OFFSET (m)	0.000	6.050	6.500	6.692	11.000	11.800	14.621	

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<u>CH 140.00</u>

<u>CH 100.00</u>

<u>CH 80.00</u>

<u>CH 220.00</u>

							SITE LEV	EL	
———————		-3.0%			4.0% -	3.0%	% ───── -25.0%		
DATUM R.L. 40.00								7	
DESIGN SURFACE (mAHD)	42.951	42.769	42.742	42.967	43.140	43.116	41.214	41.207	
EXISTING SURFACE (mAHD)	42.887	41.856	41.820	41.805	41.531	41.505	41.214	41.213	
EVEL DIFFERENCE (m)	0.064	0.913	0.922	1.163	1.609	1.611	0.000	-0.006	
DFFSET (m)	0.000	6.050	6.500	6.692	11.000	11.800	19.406	19.445	
							· ·		

<u>CH 200.00</u>

 $_{/}$ SITE LEVEL

	│ WORKS E │ (TYP)	BY OTHERS								
———————		-3.0%	ſ		4.0% -3	3.0%	~25.0%			
DATUM R.L. 40.00										
DESIGN SURFACE (mAHD)	43.397	43.216	43.189	43.414	43.586	43.562	42.379	42.341	42.323	
EXISTING SURFACE (mAHD)	43.403	42.936	42.895	42.878	42.680	42.648	42.379	42.362	42.354	
LEVEL DIFFERENCE (m)	-0.005	0.280	0.294	0.536	0.906	0.914	000.0	-0.021	-0.031	
OFFSET (m)	0.000	6.050	6.500	6.692	11.000	11.800	16.533	16.796	16.926	

<u>CH 180.00</u>

DATUM R.L. 42.00	
DESIGN SURFACE (mAHD)	
EXISTING SURFACE (mAHD)	
LEVEL DIFFERENCE (m)	
OFFSET (m)	

<u>CH 160.00</u>

								_ SITE LEVEL
		-3.0%			4.0% -	3.0%	⁶ -25.0%	
DATUM R.L. 42.00								
DESIGN SURFACE (mAHD)	45.063	44.881	44.854	45.079/	45.252	45.228	44.528	
EXISTING SURFACE (mAHD)	45.026	44.695	44.671	44.661	44.463	44.422	44.280	
LEVEL DIFFERENCE (m)	0.037	0.186	0.183	0.418	0.789	0.805	0.248	
OFFSET (m)	0.000	6.050	6.500	6.692	11.000	11.800	14.600	

NOT FOR CONSTRUCTION

	Scale at A1	Drawn	Designed	Approved						
SECTIONS	1:200	ES	AW	CR						
SIREEI	Project No	Originator	Туре	Role Sheet No.	Rev					
	STHS-TTW-01-00-DR-C-01302-3									
	10.01.2025 3:16 PM									

3	SCHEMATIC DESIGN FOR REF	JL ES 10.01.2025	5				
2	SCHEMATIC DESIGN FOR REF	JL ES 06.12.2024					
1	FINAL DRAFT ISSUE FOR REF	JL ES 21.11.2024					
Rev	Description	Eng Draft Date	Rev Description	Eng Draft Date	Rev Description	Eng Draft	Date

0.0	4.0	8.0	12.0	16.0m
1:200 /	A1		1:4	400 A3

		-3.0%			
DATUM R.L. 40.00					
DESIGN SURFACE (mAHD)	42.426	42.245	42.218	42.443/	
EXISTING SURFACE (mAHD)	42.455	42.097	42.074	42.064	
LEVEL DIFFERENCE (m)	-0.029	0.148	0.144	0.378	
OFFSET (m)	0.000	6.050	6.500	6.692	

✓ WATER FILLED BARRIER

CH 2	080	00

	└ MOKKS I	BY OTHERS						
—————————		-3.0%			4.0% -3	3.0%	~25.0%	
DATUM R.L. 40.00			_ _					
DESIGN SURFACE (mAHD)	42.350	42.169	42.142	42.367	42.539	42.515	40.966	
EXISTING SURFACE (mAHD)	41.679	41.446	41.427	41.419	41.208	41.178	40.966	
LEVEL DIFFERENCE (m)	0.672	0.723	0.715	0.948	1.331	1.337	0.000	
OFFSET (m)	0.000	6.050	6.500	6.692	11.000	11.800	17.996	

СН	260.00

							SITE LEVEL	\neg
		-3.0%			4.0% -3	3.0%	% 	
DATUM R.L. 40.00								
DESIGN SURFACE (mAHD)	42.550	42.369	42.342	42.567 /	42.739	42.715	40.345	
EXISTING SURFACE (mAHD)	41.462	41.094	41.063	41.050	40.828	40.777	40.297	
LEVEL DIFFERENCE (m)	1.088	1.275	1.279	1.517	1.911	1.938	0.048	
OFFSET (m)	0.000	6.050	6.500	6.692	11.000	11.800	21.281	

<u>CH 240.00</u>

DATUM R.L. 40.00 0000 DESIGN SURFACE (mAHD) 0000 EXISTING SURFACE (mAHD) 0000 986 45.243 1.307 40.648 45.243 45.243 1.317 11.000 0000 0000			-3.0%			4.0%		
DESIGN SURFACE (mAHD) (mAHD) EXISTING SURFACE (mAHD) 42.569 1.916 40.663 42.542 2.341 40.648 42.542 2.341 40.648 42.767 OFFSET (m) 000 000	DATUM R.L. 40.00						 	
EXISTING SURFACE (mAHD) 988 - - Level 136 136 138 137 131 137 131 138 190 139 190 130 190 131 190 132 131 131 1000 132 131 131 1000 132 131 131 1000 132 1000 132 1000 133 1000 140 1000 150 1000 160 1000 17 1000 180 1000 190 1000 190 1000 190 1000 190 1000 190 1000 190 1000 190 1000 190 1000 190 1000 190 1000 190 1000	DESIGN SURFACE (mAHD)	42.750	42.569	42.542	42.767	42.936		
LEVEL DIFFERENCE (m) 1365 1366 1366 1367 <th 1367<<="" td=""><td>EXISTING SURFACE (mAHD)</td><td>40.886</td><td>40.653</td><td>40.648</td><td>40.646</td><td>40.596</td><td></td></th>	<td>EXISTING SURFACE (mAHD)</td> <td>40.886</td> <td>40.653</td> <td>40.648</td> <td>40.646</td> <td>40.596</td> <td></td>	EXISTING SURFACE (mAHD)	40.886	40.653	40.648	40.646	40.596	
OFFSET (m) 000 000 000 000	LEVEL DIFFERENCE (m)	1.865	1.916	1.894	2.121	2.341		
	OFFSET (m)	0.000	6.050	6.500	6.692	10.925		

 $_{
m \Gamma}$ SITE LEVEL

NOT FOR CONSTRUCTION

Client:

School Infrastructure NSW

Engineer:

NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG

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	Scale at A1	Drawn
SS SECTIONS	1:200	ES
AL STREET	Project No	Originator
T2	STHS	-TTW-

1:200	ES	AW	CR	
Project No	Originator	Туре	Role Sheet No.	Rev
STHS	S-TTW-01	-00-DR-	-C-01303	3-3
10.01.20	25 3:17 PM			

Designed

Approved

1000

2000

3000

4000mm

School Infrastructure NSW

NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG

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3500	
VERGE/ PATH	
NN EXISTING ROLL KERB	ROAD BOUNDARY

NOTE: ROAD AND FOOTPATH CROSSFALLS TO BE IN ACCORDANCE WITH BLACKTOWN CITY COUNCIL'S REQUIREMENTS.

	Scale at A1	Drawn	Designed	Approved	
NS	1:50	ES	AW	CR	
	Project No	Originator	Туре	Role Sheet No.	Rev
	STHS	-TTW-01-	00-DR-	-C-01401	-3
	10.01.20	25 3:17 PM			

3	SCHEMATIC DESIGN FOR REF	JL ES 10.01.2025	5					
2	SCHEMATIC DESIGN FOR REF	JL ES 06.12.2024						
1	FINAL DRAFT ISSUE FOR REF	JL ES 21.11.2024						
Rev	Description	Eng Draft Date	Rev Description	Eng Draft	Date	Rev Description	Eng Draft	Date

School Infrastructure NSW

Engineer

NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG

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NOTE: ROAD AND FOOTPATH CROSSFALLS TO BE IN ACCORDANCE WITH BLACKTOWN CITY COUNCIL'S REQUIREMENTS.

	Scale at A1	Drawn	Designed	Approved	
TIONS	1:50	ES	AW	CR	
	Project No	Originator	Туре	Role Sheet No.	Rev
	STHS	G-TTW-01	-00-DR	-C-01402	2-3
	10.01.20	025 3:17 PM			

Eng Draft Date Rev Description Eng Draft Date Rev Description Eng Draft Date Rev Description

EROSION AND SEDIMENT CONTROL PUMP OUT NOTES

ANY ACCUMULATED WATER CONTAMINATED WITH SEDIMENT, FROM A SEDIMENT BASIN OR EXCAVATION PIT, IS TO BE FLOCCULATED OR FILTERED IN ORDER TO LOWER THE SUSPENDED SOLID LOAD TO LESS THAN 50MG PER LITRE GYPSUM GAS OR OTHER APPROVED FLOCCULANT SHOULD BE APPLIED WITHIN 24 HOURS OF THE END OF THE STORM EVENT. THE GYPSUM MUST BE SPREAD EVENLY OVER THE ENTIRE WATER SURFACE. PUMPING IS NOT TO OCCUR FOR AT LEAST 36 HOURS AND PREFERABLY 48 HOURS AFTER APPLICATION. CLEAN WATER IS TO BE DISCHARGED TO THE WATER TABLE VIA A HALE BAIL SEDIMENT FILTER IN A WAY THAT DOES NOT PICK UP SEDIMENT THAT HAS DROPPED TO THE BOTTOM. NOTE: GYPSUM IS A HYDRATED FORM OF CALCIUM SULPHATE AND IS AVAILABLE AT MANY SWIMMING POOL SHOPS AND HARDWARE STORES.

DISTURBED AREA

TEMPORARY CONSTRUCTION VEHICLE EXIT NTS

EXISTING SURFACE LEVEL

SANDBAG KERB INLET SEDIMENT TRAP NTS

Rev	Description	Eng Draft Date	Rev Description	Eng Draft Date	Rev Description	Eng Draft	Date
1	FINAL DRAFT ISSUE FOR REF	JL ES 21.11.2024					
2	SCHEMATIC DESIGN FOR REF	JL ES 06.12.2024					
3	SCHEMATIC DESIGN FOR REF	JL ES 10.01.2025					

TYPICAL SECTION THROUGH CATCH DRAIN SCALE 1:20

Enginee

NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG

NOT FOR CONSTRUCTION

	Scale at A1	Drawn	Designed	Approved	
N AND SEDIMENT		ES	AW	CR	
OL NOTES AND	Project No	Originator	Туре	Role Sheet No.	Rev
)	STHS	-TTW-01 [,]	-00-DR-	-C-02001	-3
	10.01.202	25 3:18 PM			

(i) it complies with the specification requirements for fill material, (ii) the placement moisture content complies with the Geotechnical Consultants requirements, and allows filling to be placed and proofrolled in accordance with the specification. Where necessary the Contractor must moisture condition the excavated material to meet these requirements.

4. Compact fill areas and subgrade to not less than:

Location	Standard d (AS 1289 5	Iry density 5.1.1.) ((Moisture OMC)	
Under building slabs Under roads and carp Landscaped areas:	on ground: barks:	98% 98% 95%	±2% ±2% ±2%	

 Before placing fill, proof roll exposed subgrade with a 12 tonne minimum roller to test subgrade and then remove soft spots(areas with more than 3mm movement under roller). Soft spots to be replaced with granular fill U.N.O. 6. Contractor shall place safety barriers around excavations in accordance with

No.	FROM LEVEL (m)	TO LEVEL (m)	COLOUR
1	-3.00	-2.00	
2	-2.00	-1.00	
3	-1.00	0.00	
4	0.00	1.00	
5	1.00	2.00	
6	2.00	3.00	
7	3.00	4.00	
8	4.00	5.00	

relevant safety regulations.

- earthworks drawings refer to the bulk earthworks construction legend.
- 10. Detailed earthworks such as piling, pile caps, ground beams, lift pits, service
- calculations:
- is <u>4563</u>m³. Bulk earthworks level
- 500mm below finished floor level (buildings)

- for commercial and residential development.

NOTE: WORKS SHOWN TO GUNTAWONG ROAD ARE SUBJECT TO SEPARATE APPROVAL (NOT UNDER THIS REF)

Rev	Description	Eng	Draft	Date	Rev	Description	Eng Draft	Date	Rev De	scription	Eng Draft	Dat
1	FINAL DRAFT ISSUE FOR REF	JL	ES 2	1.11.2024								
2	SCHEMATIC DESIGN FOR REF	JL	ES 0	6.12.2024								
3	SCHEMATIC DESIGN FOR REF	JL	ES 1	0.01.2025								
4	SCHEMATIC DESIGN FOR REF	JL	ES 2	2.01.2025								

Cut	Fill	Net
6723 Cu. M.	12446 Cu. M.	5723 Cu. M. <fill></fill>

STORMWATER DRAINAGE

1.	STORMWATER DESIGN CRITERIA						
	 (A) AVERAGE EXCEEDANCE PROBABILITY: - 1% AEP FOR ROOF DRAINAGE TO FIRST EXTERNAL PIT 5% AEP FOR PAVED AND LANDSCAPED AREAS 						
	 (B) RAINFALL INTENSITIES : - TIME OF CONCENTRATION: 5 MINUTES 1% AEP = 235mm/hr 5% AEP = 177mm/hr 						
	(C) RAINFALL LOSSES: - - IMPERVIOUS AREAS: IL = 1.5mm CL = 0mm/hr - PERVIOUS AREAS: IL = 28mm CL = 1.2mm/hr						
2.	PIPES 300 DIA AND LARGER TO BE REINFORCED CONCRETE CLASS "4" APPROVED SPIGOT AND						
3.	PIPES UP TO 300 DIA MAY BE SEWER GRADE UPVC WITH SOLVENT WELDED JOINTS, SUBJECT TO APPROVAL BY THE ENGINEER						

- 4. EQUIVALENT STRENGTH VCP OR FRP PIPES MAY BE USED SUBJECT TO APPROVAL
- 5. PRECAST PITS MAY BE USED EXTERNAL TO THE BUILDING SUBJECT TO APPROVAL BY ENGINEER.
- 6. ENLARGERS, CONNECTIONS AND JUNCTIONS TO BE MANUFACTURED FITTINGS WHERE PIPES ARE LESS THAN 300 DIA.
- 7. WHERE SUBSOIL DRAINS PASS UNDER FLOOR SLABS AND VEHICULAR PAVEMENTS, UNSLOTTED UPVC SEWER GRADE PIPE IS TO BE USED.
- GRATES AND COVERS SHALL CONFORM WITH AS 3996-2006, AND AS 1428.1 FOR ACCESS 8.
- REQUIREMENTS. 9. PIPES ARE TO BE INSTALLED IN ACCORDANCE WITH AS 3725. ALL BEDDING TO BE TYPE H2 U.N.O. 10. CARE IS TO BE TAKEN WITH INVERT LEVELS OF STORMWATER LINES. GRADES SHOWN ARE NOT TO
- BE REDUCED WITHOUT APPROVAL. 11. ALL STORMWATER PIPES TO BE 150 DIA AT 1.0% MIN FALL U.N.O.
- 12. SUBSOIL DRAINS TO BE SLOTTED FLEXIBLE UPVC U.N.O.
- 13. ADOPT INVERT LEVELS FOR PIPE INSTALLATION (GRADES SHOWN ARE ONLY NOMINAL).

STORMWATER PIPE INFORMATION

PIPE LENGTH

PIPE GRADE

PIPE INFORMATION

TIE	INFORMATION

SW

Ø150

L 10.0m TIE LENGTH D 1.0m TIE DEPTH

TIE DIAMETER

STORMWATER STRUCTURE IDENTIFICATION

DOWNSTREAM INVERT LEVEL

UPSTREAM INVERT LEVEL

PIPE INTERNAL DIAMETER

HYDRAULIC FLOW RATE

PIPE MATERIAL AND CLASS

SW1-2

USIL

Ø000

--- '-' 0.0m

0.0 m/s

%0.0

DSIL

LINE NUMBER 1 - STRUCTURE NUMBER 2

WHERE SUBSOIL DRAINS PAS uPVC SEWER GRADE PIPE IS
SUBSOIL DRAINS TO BE Ø100

SUBSOIL DRAINAGE

2.

3.

- 4. ALL SUBSOIL DRAINS ARE TO BE AT MINIMUM 1% GRADE UNLESS NOTED OTHERWISE.
- 5. ALL SUBSOIL DRAINS TO BE RODDED PRIOR TO THE PLACEMENT OF ASPHALT.
- 6. ALL SUBSOIL DRAINS ARE DRAWN DIAGRAMMATICALLY FOR CLARITY. REFER TO TYPICAL DETAIL FOR SUBSOIL SETOUT.

STORMWATER LEGEND

	STORMWATER PIPE
• ^{DP}	DOWN PIPE
° ^{RP}	RODDING POINT
• ^{PO}	PLANTER OUTLET
o RO	RAINWATER OUTLET
GPT	GROSS POLLUTANT T
	OVERLAND FLOW ARR
	CONCRETE INCASED F
>	SWALE DRAIN

STORMWATER ANNOTATIONS

IL	PIPE INVERT LEVEL
OL	PIPE OBVERT LEVEL
CL	PIT COVER LEVEL
WL	WATER LEVEL
CL WL	PIT COVER LEVEL

<u>NOTE</u>

STORMWATER DRAINAGE NOTES AND LEGEND IS TO READ IN CONJUNCTION WITH GENERAL NOTES AND LEGEND. REFER DRAWING No. 00002

SCALE 1:50

GRATED INLET SUMP SCALE 1:50

3 SCHEMATIC DESIGN FOR REF JL ES 10.01.2025 2 SCHEMATIC DESIGN FOR REF JL ES 06.12.2024 1 FINAL DRAFT ISSUE FOR REF JL ES 21.11.2024 Eng Draft Date Rev Description Eng Draft Date Rev Description Eng Draft Date Rev Description

- 1. ALL SUBSOIL DRAINAGE WORKS ARE TO BE COMPLETED IN ACCORDANCE WITH THE RELEVANT STANDARDS AND SPECIFICATIONS OUTLINED IN THE PROJECT SPECIFICATION.
 - SS UNDER FLOOR SLABS AND VEHICULAR PAVEMENTS UNSLOTTED TO BE USED.
 - SLOTTED FLEXIBLE uPVC UNLESS NOTED OTHERWISE.

DESIGN INVERT LEVELS

SCALE 1:20

Inginee

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NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG

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KERB INLET STRUCTURE (KIS) COVER LEVEL FOR KIS IN ROAD SCALE 1:20

	Scale at A1	Drawn	Designed	Approved			
IWATER		ES	AW	CR			
AND LEGEND	Project No	Originator	Туре	Role Sheet No.	Rev		
1	STHS-TTW-01-00-DR-C-04001-3						
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38.0	0 1.100 A1		1:200 A3	-H		38.24
		\sum	38.12			
		PP				
						39.00
						30
37.74						38.
		20.00			38.0	38.17
		39.00	21.67		37.68	
		-38.00-				
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31.28	31.31		97		37_06	31.
					36.0"	37/2
		DISPERSION TRENCH	/			3139
37.10	SURCHARGE PIT]	11	37.20	31,23	
	36.85		36.1	37.19	37.10 37.20	ol 31.30
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36.86	26 ⁵⁶ . 68		30.	36.' 30	gr /	
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3 SCHEMATIC DESI	GN FOR REF JL ES 10.01.2025					
1 FINAL DRAFT ISS	JE SOUTOR REF JE ES 06.12.2024 JE JE <thje< thr=""></thje<>					
Rev Description	Eng Draft Date R	ev Description	Eng	Draft Date	Rev Description	Eng Draft Date

	Scale at A1	Drawn	Designed	Approved			
	1:250	ES	AW	CR			
JBSOIL DRAINAGE	Project No	Originator	Туре	Role Sheet No.	Rev		
SHEET 4	STHS-TTW-01-00-DR-C-04104-4						
	22.01.20	25 11:46 AN	1				

4	SCHEMATIC DESIGN FOR REF	JL ES 22.01.2025				
3	SCHEMATIC DESIGN FOR REF	JL ES 10.01.2025				
2	SCHEMATIC DESIGN FOR REF	JL ES 06.12.2024				
1	FINAL DRAFT ISSUE FOR REF	JL ES 21.11.2024				
Rev	Description	Eng Draft Date	Rev Description	Eng Draft Date	Rev Description	Eng Draft Date

School Infrastructure NSW

NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG

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- SITE BOUNDARY

– SURCHARGE PIT AND ROCK FILLED INFILTRATION TRENCH

		LEGEND			
402				PROPOSED RETAININ	G WALL
""," ⁰ ,					
1					1
1,85 41.65		1.2.71	42. ⁸¹		15
	42.05	м.			5
			L2.71		
				42.95	
	41.81	42 lyle	1×2.81		
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10.86					
h1. ⁵² h ^{1,64}					
jî					
8					

Drawing Title: RETAINING WALL PLAN SHEET 2 Scale at A1 Drawn Designed Approved ES CR Project No Originator Type Role Sheet No. Rev STHS-TTW-01-00-DR-C-06102-3 10.01.2025 3:25 PM

NOTE: RETAINING WALLS ARE SHOWN INDICATIVELY AND ARE SUBJECT TO DETAILED DESIGN

Eng Draft Date Rev Description

Rev Description

Eng Draft Date

Eng Draft Date Rev Description

NOT FOR CONSTRUCTION

TALLAWONG

- COMPACTED SELECT BACKFILL IN

ACCORDANCE WITH AS 3798 AND THE SPECIFICATION

School Infrastructure NSW

Client:

NSW

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10.01.2025 3:25 PM

STHS-TTW-01-00-DR-C-06501-3

CONCRETE

1. PLACE CONCRETE OF THE FOLLOWING CHARACTERISTIC COMPRESSIVE STRENGTH fc IN

ACCORDANCE WITH AS 1379. 2

LOCATION	f'c MPa (28 DAYS)	SPECIFIED SLUMP	NOMINAL AGG. SIZE
KERBS	S20	80	20
RETAINING WALL FOOTINGS	S40	80	20

- 3. USE TYPE 'GP' CEMENT, UNLESS OTHERWISE SPECIFIED.
- 4. ALL CONCRETE SHALL BE SUBJECT TO PROJECT ASSESSMENT AND TESTING TO AS 1379. CONSOLIDATE BY MECHANICAL VIBRATION. CURE ALL CONCRETE SURFACES AS DIRECTED IN THE SPECIFICATION.
- 6. FOR ALL FALLS IN SLAB, DRIP GROOVES, REGLETS, CHAMFERS ETC. REFER TO ARCHITECTS
- DRAWINGS AND SPECIFICATIONS. UNLESS SHOWN ON THE DRAWINGS, THE LOCATION OF ALL CONSTRUCTION JOINTS SHALL BE
- SUBMITTED TO ENGINEER FOR REVIEW.
- NO HOLES OR CHASES SHALL BE MADE IN THE SLAB WITHOUT THE APPROVAL OF THE ENGINEER 9. CONDUITS AND PIPES ARE TO BE FIXED TO THE UNDERSIDE OF THE TOP REINFORCEMENT LAYER.
- 10. SLURRY USED TO LUBRICATE CONCRETE PUMP LINES IS NOT TO BE USED IN ANY STRUCTURAL MEMBERS.
- 11. ALL SLABS CAST ON GROUND REQUIRE SAND BLINDING WITH A CONCRETE UNDERLAY

CONCRETE FINISHING

- 1. ALL EXPOSED CONCRETE PAVEMENTS ARE TO BE BROOMED FINISHED. 2. ALL EDGES OF THE CONCRETE PAVEMENT INCLUDING KEYED AND DOWELLED JOINTS ARE TO BE
- FINISHED WITH AN EDGING TOOL. 3. CONCRETE PAVEMENTS WITH GRADES GREATER THAN 10 % SHALL BE HEAVILY BROOMED
- FINISHED.
- 4. CARBORUNDUM TO BE ADDED TO ALL STAIR TREADS AND RAMPED CROSSINGS U.N.O.

FORMWORK

1. THE DESIGN, CERTIFICATION, CONSTRUCTION AND PERFORMANCE OF THE FORMWORK, FALSEWORK AND BACKPROPPING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. PROPOSED METHOD OF INSTALLATION AND REMOVAL OF FORMWORK IS TO BE SUBMITTED TO THE SUPERINTENDENT FOR COMMENT PRIOR TO WORK BEING CARRIED OUT.

PAVEMENT LEGEND

PT1	CONCRETE BLEACHERS
PT2	INTERNAL PEDESTRIAN PAVING 120mm THICK CONCRETE SLAB (25MPa) WITH SL72 MESH (40 COVER) 150mm THICK COMPACTED FINE CRUSHED ROCK (DGB20) REFER TO LANDSCAPE ARCHITECTS DOCUMENTATION FOR COLOUR TREATMENT
PT3	CARPARK AND DELIVERY ZONE 40mm COMPACTED THICKNESS AC14 WEARING COURSE ON 150mm COMPACTED THICKNESS DGB20 CLASS 1 BASE TO 98% MMDD AT ±2% OMC ON 175mm COMPACTED THICKNESS DGS20 SUBBASE TO 98% MMDD AT ±2% OMC ON SUBGRADE MIN. CBR 4% COMPACTED TO 98% SMDD AT ±2% OMC
PT4	MULTI SPORTS COURTS TO LANDSCAPE ARCHITECT'S DOCUMENTATION
PT5	SPORTS FIELD TO LANDSCAPE ARCHITECT'S DOCUMENTATION
PT6	INTERNAL PEDESTRIAN PAVING 120mm THICK CONCRETE SLAB (25MPa) WITH SL72 MESH (40 COVER) 150mm THICK COMPACTED FINE CRUSHED ROCK (DGB20) REFER TO LANDSCAPE ARCHITECTS DOCUMENTATION FOR COLOUR TREATMENT
PT8	PUBLIC DOMAIN REINFORCED CONCRETE DRIVEWAY 150 THICK S32 CONCRETE 150 THICK DGB20 COMPACTED TO 98% MMDD
PT20	PUBLIC DOMAIN ROAD MILL AND RESHEET 2x25mm THICK WEARING COURSE AC10 PRIME AC00 EXISTING PAVEMENT
PT21	PUBLIC DOMAIN ROAD PAVEMENT REFER DRAWING 07501
PT22	PUBLIC DOMAIN FOOTPATH 125mm THICK CONCRETE SLAB (25MPa) WITH SL72 MESH (40 COVER) 150mm THICK COMPACTED FINE CRUSHED ROCK (DGB20)
	LANDSCAPING REFER TO LANDSCAPE ARCHITECT'S DOCUMENTATION
NOTES: 1. PAVEMENT 2. ADOPTED D	BUILDUPS ARE INDICATIVE AND TO BE DEVELOPED IN DETAILED DESIGN. DESIGN PARAMETERS:

DESIGN TRAFFIC 5x10° ESA, SUBGRADE 4% CBR MIN.

SYMBO N

RL

LO

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SL

FOOTINGS WALLS

FABRIC LAP

•	

TENSION LAPS									
BAR SIZE	TOP BARS IN BANDS AND BEAMS	ALL OTHER BARS							
N12	570	480							
N16	800	700							
N20	1150	950							
N24	1500	1250							
N28	1850	1500							
N32	2250	1800							
N36	2700	2100							

COMPRESSION LAPS BAR SIZE 640 N16 640 N20 800				
BAR SIZE				
N16	640			
N20	800			
N24	960			
N28	1120			
N32	1280			
N36	1440			

ASSUMPTIONS: 1. TOP BARS IN BANDS AND BEAMS:

- MORE THAN 300mm OF CONCRETE BELOW THE BAR.
- FOR 32Mpa CONCRETE,
- 3. f'c = 32Mpa ALL OTHER BARS:
- BARS = $2 \times Cd = 50 \text{mm}$.
- 3. f'c = 32Mpa. COLUMNS:

3	SCHEMATIC DESIGN FOR REF	JL	ES	10.01.2025						
2	SCHEMATIC DESIGN FOR REF	JL	ES	06.12.2024						
1	FINAL DRAFT ISSUE FOR REF	JL	ES	21.11.2024						
Rev	Description	Eng	Draft	Date	Rev Description	Eng Draft	Date	Rev Description	Eng Draft	Date

NOT FOR CONSTRUCTION

CONCRETE REINFORCEMENT

1. FIX REINFORCEMENT AS SHOWN ON DRAWINGS. THE TYPE AND GRADE IS INDICATED BY A SYMBOL AS SHOWN BELOW. ON THE DRAWINGS THIS IS FOLLOWED BY A NUMERAL WHICH INDICATES THE SIZE IN MILLIMETRES OF THE REINFORCEMENT.

L	TYPE	GRADE
	HOT ROLLED RIBBED BAR	DN500N
	PLAIN ROUND BAR	R250N
	SQUARE MESH	500L
	RECTANGULAR MESH	500L

2. PROVIDE BAR SUPPORTS OR SPACERS TO GIVE THE FOLLOWING CONCRETE COVER TO ALL REINFORCEMENT UNLESS OTHERWISE NOTED ON DRAWINGS.

CATION	COVER (MM)
	50
	30

3. COVER TO REINFORCEMENT ENDS TO BE 50 mm U.N.O.

4. PROVIDE N12-450 SUPPORT BARS TO TOP REINFORCEMENT AS REQUIRED, LAP 500 U.N.O. 5. MAINTAIN COVER TO ALL PIPES, CONDUITS, REGLETS, DRIP GROOVES ETC

6. ALL COGS TO BE STANDARD COGS UNLESS NOTED OTHERWISE. 7. FABRIC END AND SIDE LAPS ARE TO BE PLACED STRICTLY IN ACCORDANCE WITH THE

MANUFACTURERS REQUIREMENTS TO ACHIEVE A FULL TENSILE LAP. FABRIC SHALL BE LAID SO THAT THERE IS A MAXIMUM OF 3 LAYERS AT ANY LOCATION.

8. LAPS IN REINFORCEMENT SHALL BE MADE ONLY WHERE SHOWN ON THE DRAWINGS UNLESS OTHERWISE APPROVED. LAP LENGTHS AS PER TABLE BELOW.

2. MINIMUM COVER OF 25mm AND MINIMUM STIRRUP SIZE OF N12 GIVING Cd=37mm; THEREFORE MINIMUM CLEAR SPACING BETWEEN BARS = 2 X Cd = 74mm. MINIMUM COVER IS BASED ON THE NEW A2 EXPOSURE CLASSIFICATION FOR INTERIOR, NON-RESIDENTIAL WHICH REQUIRES 25mm COVER

1. LESS THAN 300mm OF CONCRETE BELOW THE BAR.

2. MINIMUM COVER OF 25mm GIVING Cd = 25mm; THEREFORE MINIMUM CLEAR SPACING BETWEEN

1. COVER TO COLUMNS = 40mm (30+10)k7 = 1.25 2. COVERS FOR FIRE RATING ARE TO BE DESIGNED BY THE ENGINEER.

VEHICULAR PAVEMENT JOINTING (03000 SERIES DRAWINGS)

- 1. ALL VEHICULAR PAVEMENTS TO BE JOINTED AS SHOWN ON DRAWINGS.
- 2. DOWEL BARS ARE TO BE IN ACCORDANCE WITH GIVEN DETAIL. REFER 03000 SERIES DRAWINGS. 3. DOWELED EXPANSION JOINTS SHOULD GENERALLY BE LOCATED AT A MAXIMUM OF 24.0M CENTRES.
- 4. SAWN JOINTS SHOULD GENERALLY BE LOCATED AT A MAXIMUM OF 6.0M CENTRES OR 1.5 X THE SPACING OF PERPENDICULAR SAWN JOINTS.
- 5. PROVIDE 10mm WIDE FULL DEPTH EXPANSION JOINTS BETWEEN BUILDINGS/STRUCTURES AND ALL CONCRETE OR UNIT PAVERS.
- 6. THE TIMING OF THE SAW CUT IS TO BE CONFIRMED BY THE CONTRACTOR ON SITE. SITE CONDITIONS WILL DETERMINE HOW MANY HOURS AFTER THE CONCRETE POUR BEFORE THE SAW CUTS ARE COMMENCED. REFER TO THE SPECIFICATION FOR WEATHER CONDITIONS AND TEMPERATURES REQUIRED.
- 7. VEHICULAR PAVEMENT JOINTING AS FOLLOWS.

PEDESTRIAN PATH JOINTING (03000 SERIES DRAWINGS)

- 1. EXPANSION JOINTS ARE TO BE LOCATED WHERE POSSIBLE AT TANGENT POINTS OF CURVES AND
- ELSEWHERE AT MAX 6.0M CENTRES. 2. WEAKENED PLANE JOINTS ARE TO BE LOCATED AT A MAX 1.5 X WIDTH OF THE PAVEMENT.
- 3. WHERE POSSIBLE JOINTS SHOULD BE LOCATED TO MATCH KERBING AND / OR ADJACENT PAVEMENT JOINTS.
- 4. ALL PEDESTRIAN FOOTPATH JOINTING AS FOLLOWS (UNO).

6.0m N	MAX SPACIN	G	- 6	8.0m MA
 - — EXPANS	SION JOINT	JOINT		1.5xW

<u>KERBING</u>

INCLUDES ALL KERBS, GUTTERS, DISH DRAINS, CROSSINGS AND EDGES.

- 1. ALL KERBS, GUTTERS, DISH DRAINS AND CROSSINGS TO BE CONSTRUCTED ON MINIMUM 75mm GRANULAR BASECOURSE COMPACTED TO MINIMUM 98% MODIFIED MAXIMUM DRY DENSITY IN
- ACCORDANCE WITH AS 1289 5.2.1. 2. EXPANSION JOINTS (EJ) TO BE FORMED FROM 10mm COMPRESSIBLE CORK FILLER BOARD FOR THE FULL DEPTH OF THE SECTION AND CUT TO PROFILE. EXPANSION JOINTS TO BE LOCATED AT DRAINAGE PITS, ON TANGENT POINTS OF CURVES AND ELSEWHERE AT 12M CENTRES EXCEPT FOR
- INTEGRAL KERBS WHERE THE EXPANSION JOINTS ARE TO MATCH THE JOINT LOCATIONS IN SLABS. 3. WEAKENED PLANE JOINTS TO BE MIN 3mm WIDE AND LOCATED AT 3M CENTRES EXCEPT FOR INTEGRAL KERBS WHERE WEAKENED PLANE JOINTS ARE TO MATCH THE JOINT LOCATIONS IN
- SLABS. 4. BROOMED FINISHED TO ALL RAMPED AND VEHICULAR CROSSINGS, ALL OTHER KERBING OR DISH DRAINS TO BE STEEL FLOAT FINISHED.
- 5. IN THE REPLACEMENT OF KERBS EXISTING ROAD PAVEMENT IS TO BE SAWCUT 900mm FROM LIP OF GUTTER. UPON COMPLETION OF NEW KERBS, NEW BASE COURSE AND SURFACE IS TO BE LAID 900mm WIDE TO MATCH EXISTING MATERIALS AND THICKNESSES. EXISTING ALLOTMENT DRAINAGE PIPES ARE TO BE BUILT INTO THE NEW KERB WITH A 100mm DIA HOLE. EXISTING KERBS ARE TO BE COMPLETELY REMOVED WHERE NEW KERBS ARE SHOWN.

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Drawing Title:

	Scale at A1	Drawn	Designed	Approved	
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2 SCHEMATIC DESIGN FOR REF JL ES 06.12.2024

1 FINAL DRAFT ISSUE FOR REF JL ES 21.11.2024

Rev Description

Eng Draft Date Rev Description

SCHOFIELDS TALLAWONG

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CONCRETE THICKNESS	DOWEL SIZE	DOWEL LENGTH (L)
150 - 190	Ø20	450
200 - 240	Ø24	450
250 - 270	Ø30	450
280 - 340	Ø33	450
>340	Ø36	500

DOWEL JOINT DETAIL SCALE 1:5

Inginee

NOTES

- PROVIDE TRIMMER BARS AT SLAB PENETRATIONS (COLUMNS, WALLS, PITS ETC). 1
- ALL TRIMMER BARS TO BE 1N16 U.N.O 2.
- PROVIDE 50 COVER EACH FACE U.N.O 3.
- 4. TRIMMER DETAILS ARE TO BE READ IN CONJUNCTION WITH GIVEN JOINTING DETAILS

TYPICAL SLAB ON GROUND TRIMMER BAR DETAILS

3	SCHEMATIC DESIGN FOR REF	JL ES 10.01.2)25						
2	SCHEMATIC DESIGN FOR REF	JL ES 06.12.2)24						
1	FINAL DRAFT ISSUE FOR REF	JL ES 21.11.2)24						
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	2	SCHEMATIC DESIGN FOR REF	JL	ES	06	6.12.2024						
	3	SCHEMATIC DESIGN FOR REF	JL	ES	10	.01.2025						
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NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG

Drawing Title: PAVEME DETAIL

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1	FINAL DRAFT ISSUE FOR REF	JL	ES 21.11.2024						
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3000 4000mm

100 A3

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Engineer

NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG

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Rev	Description	Eng	Draft	Date	Rev	Description	Eng Draft	Date	Rev Description	Eng Draft	Date

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STAIRS

NOTES

2.

ILLUSTRATE THEIR POSITION IN RELATION TO THE STAIRS IN ACCORDANCE WITH AS1428.1. FOR DETAILS ON THE TYPE AND COLOUR OF TACTILE INDICATORS, REFER LANDSCAPE OR ARCHITECTURAL DOCUMENTATION (TYPICAL)

- TACTILE INDICATORS ARE SHOWN TO

REFER 01000 SERIES DRAWINGS FOR LEVELS

1:4 BATTER TO EXISTING.

— TACTILE INDICATORS ARE SHOWN TO ILLUSTRATE THEIR POSITION IN RELATION TO THE STAIRS IN ACCORDANCE WITH AS1428.1. FOR DETAILS ON THE TYPE AND COLOUR OF TACTILE INDICATORS, REFER LANDSCAPE OR ARCHITECTURAL DOCUMENTATION (TYPICAL)

-REFER 03000 SERIES FOR PAVEMENT PROFILES AND DETAILS

REINFORCEMENT TO BE ADJUSTED TO SUIT PAVEMENT PROFILE

NSW

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NEW HIGH SCHOOL FOR SCHOFIELDS TALLAWONG

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RISER AND GOING DIMENSIONS

STAIR TYPE	RISER (R)		GOING (G)		SLOPE RELATIONSHIP (2R+G)	
	MAX	MIN	MAX	MIN	MAX	MIN
S (OTHER THAN SPRIAL)	190	115	355	240	700	550
SPIRAL	220	140	370	210	680	590

CONCRETE STRENGTH TO BE 32MPa

REFER SITE PLANS FOR SETOUT, LEVELS AND GEOMETRY 3. FOR MINIMUM SLIP RESISTANCE OF STAIR TREADS AND LANDINGS

REFER LANDSCAPE OR ARCHITECTURAL DOCUMENTATION

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PUBLIC DOMAIN)	Project No	Originator	Туре	Role Sheet No.	Rev
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Appendix B

Council Correspondence

Jema Lopez

From:	Alex Kwok <alex.kwok@blacktown.nsw.gov.au></alex.kwok@blacktown.nsw.gov.au>
То:	lema Lopez
Subject:	RE: Site Stormwater Requirements
Attachments:	OSD Deemed to Comply Tool - Developer's Edition v2.4.xlsm
Follow Up Flag: Flag Status:	Follow up Completed

You don't often get email from alex.kwok@blacktown.nsw.gov.au. Learn why this is important

[External Email]: Do not click links or open attachments unless you recognize the sender and know the content is safe.

Hi Jema

The site is classified into different zones R2 and R3 as you highlighted in red. Please go to our map online which shows the zoning of the land. BCC MapsOnline (nsw.gov.au)

In R2 zone, a development requires to provide temporary OSD and temporary water quality treatment (WQ).

The temporary OSD and WQ is required because our regional basin has not been installed yet.

In R3 zone, the development requires to provide temporary OSD and permeant water quality treatment. It because the regional basin has not been installed yet and also it was not designed to cover and treat the WQ of R3 land.

Both the requirements of the OSD and water quality treatment are showed in our WSUD Developer handbook 2020.

The reduction target you mentioned is correct.

And the OSd calculation shall be done with our spreadsheet which I attached.

If you want a further discussion, I suggest you and your client may book a pre-application meeting with us. The details of the meeting can be found in the following link: https://www.blacktown.nsw.gov.au/Plan-build/Stage-3-preparing-an-application/Pre-Application-Meetings-

nttps://www.blacktown.nsw.gov.au/Plan-build/Stage-3-preparing-an-application/Pre-Application-Meetings-PAM

Regards Alex

Alex Kwok Coordinator Drainage Development Assessment (02) 9839 6348 PO Box 63 Blacktown NSW 2148 blacktown.nsw.gov.au

We acknowledge the Dharug as the First People of the Blacktown City region

Follow us on social media

From: Jema Lopez <jema.lopez@ttw.com.au>
Sent: Tuesday, 8 October 2024 2:14 PM
To: Blacktown Council <Blacktown.Council@blacktown.nsw.gov.au>
Cc: Kirusan Subakaran <kirusan.subakaran@ttw.com.au>
Subject: RE: Site Stormwater Requirements

Good afternoon Council officer,

I just wanted to follow-up on the query below. Thank you.

Very best regards, Jema

TTW

Jema Lopez | Civil Engineer +61 2 9439 7288 | +61 2 9067 5017 | jema.lopez@ttw.com.au TTW Engineers | Sydney Read our latest news here

From: Jema Lopez Sent: Tuesday, 25 June 2024 11:25 AM To: <u>council@blacktown.nsw.gov.au</u> Subject: Site Stormwater Requirements

Good afternoon Council officer,

I hope you are well. I am seeking to confirm the stormwater requirements for a proposed educational facility at 201 Guntawong Road, Rouse Hill. The approximate extent of the site is shown in the figure below.

In regard to stormwater quantity, as per the land zoned under northwest growth area SEPP - on-site stormwater detention (OSD) requirements map snapshot shown below, it is noted that no permanent OSD is required for the site. However, temporary OSD will be required with further clarification with Council. As such, could we please obtain further clarification/confirmation on this?

In regard to the water quality requirements, the proposed development is to comply with section 4.2 Water Quality of Part J of the DCP, which states that all developments shall achieve a minimum percentage reduction of the post development average annual load of pollutants in accordance to the table below.

Table 2	Required	pollutant reduct	ion targets
		Charles I.	100 C 20 C

Pollutant	% post development average annual load reduction		
Gross pollutants	90		
Total suspended solids	85		
Total phosphorous	65		
Total nitrogen	45		
Total hydrocarbons	90		

Could you please confirm back in writing and let me know if I have missed any stormwater-related requirements or misinterpreted any of the above?

Thank you and best regards,

Jema

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